

GENERAL NOTES:

- I. GENERAL
- A. These GENERAL NOTES present and/or summarize key project information for the plans reader's convenience. See also individual PLAN NOTES and project specifications for further details and requirements.
- B. All references to reference standards herein are to the most recent issue in effect as of the date of these documents, unless noted otherwise in project specifications or on the plans.
- C. Elevations. All elevations are referenced to Ground Floor. Slab = 0'-0". All elevations shown on plans are referenced to this datum unless noted.
- D. Submit Shop Drawings, Project Data and Samples as specified in Project Specifications Section 01300.
1. Identify prominently on drawings each and all resubmittals by number.
2. Identify any changes which have been made other than those requested by the A/E.
3. Submittals failing to conform to the above will be returned for resubmittal.
- II. DESIGN CRITERIA
- A. Building Code. Arkansas Fire Prevention Code, 2012 vol II Buildings (IBC 2012 w/local amendments)
- B. Superimposed Design Loads
1. See plan notes for live and dead loads.
2. Wind:
- a. Design speed (3-sec gust) = 120 MPH (ultimate)
- b. Exposure = C
- c. Fully enclosed
3. Ground Snow: 10 psf, Importance factor = 1.1
4. Seismic:
- a. Seismic Importance Factor, I
- b. Seismic Risk Group
- c. Mapped Spectral response acceleration, S_s
- d. Mapped Spectral response acceleration, S_1
- e. Site Class
- f. Spectral response coefficient, S_{ds}
- g. Spectral response coefficient, S_{d1}
- h. Seismic design category
- i. Basic Seismic Resisting System
- Intermediate Moment Resisting Frames of Steel
- j. Design Base Shear
- k. Seismic Response Coefficient, C_s
- l. Response Modification Factor, R
- m. Analysis Procedure Used
- C. Foundations
1. See Geotechnical/Subsurface Investigation Report by Anderson Engineering Consultants, Inc. dated 4/3/2018.
2. Spread Footings: Allowable bearing pressure 2000 psf (individual column footings) or 2000 psf (continuous wall footings). Compacted subgrade density shall be at least 95% of the maximum density determined by ASTM D1557 (Modified Proctor).
3. Shallow footings exposed to weather shall bear on subgrade below the frost depth. Adjust footing elevations as needed to maintain required earth cover above the footing.
4. The contractor shall take all necessary measures to protect the subgrade below the footings and slab on grade from damage due to freeze/frost, erosion, scour and loss of bearing during construction.
5. Contractor shall not allow water to pool in excavations before or after concrete is placed. If bottoms of excavations become softened due to water from rain or other sources before footing is cast, the softened material shall be excavated and replaced with concrete or compacted fill.
- D. Future additions: none.

- III. TESTING AND INSPECTION
- A. Foundations and Earthwork. Geotechnical engineer of record/testing laboratory to be engaged by Contractor with A/E approval. See Specification Section 01400.
- B. Materials and Procedures. Testing laboratory to be engaged by Contractor with A/E approval. See specs.
- C. See spec. Section 01400 for scope of services, limits of authority, etc.
- D. Seismic Inspections and Testing: Refer to IBC, Chapter 17. Special inspections are required for this project, and shall be conducted by a certified Special Inspector.

- IV. EARTHWORK
- Review Geotechnical/Subsurface Investigation Report for complete earthwork requirements. Where the following summarized requirements conflict with the geotech report, those in the report shall be complied with.
- A. Site Preparation
1. After stripping (and excavating to the proposed subgrade level, as required), the subgrade shall be proof-rolled with a moderately heavy loaded pneumatic-tired vehicle such as a 20 to 25 ton dump truck or scraper. Soils which are observed to rut or deflect excessively under the moving load should be undercut and replaced with properly compacted fill. All prior-rolling and undercutting activities should be witnessed by the Geotechnical engineer and should be performed during a period of dry weather.
2. Within building area, compact top 6" of cut area subgrade to minimum density 95% of the maximum density by ASTM D698.
- B. Structural Fills. Select fill material compacted in 8" loose lifts.
1. Subgrade under building foundations: minimum density 95% of the maximum density by ASTM D1557.
2. Subgrade under slabs-on-grade: minimum density 95% of the maximum density by ASTM D1557.
3. Granular subbase under slab-on-grade: 8" thick clean coarse gravel (ASTM C33, Fineness modulus greater than 6) or crushed stone (#57) compacted to at least 70% of the maximum relative density by ASTM D4253 and D4254.
4. Provide vapor barrier. See specs.
- C. Groundwater Control. See geotechnical report.
- D. Utility trench excavations must be cut to competent bearing soils suitable for support of fill placement and then backfilled with properly compacted fill that is constructed as outlined in the geotechnical report.

- V. CAST-IN-PLACE REINFORCED CONCRETE: See spec. Section 03300.
- A. Design Code. ACI 318 - Strength Design. Reference Standard: ACI 301. Contractor to maintain copy at job site.
- B. Mix Design shall be documented in accordance with Section 03300 of the project specs and ACI 301 - Chapter 3 "Proportioning". Mix designs which are submitted without the required documentation will be rejected. Field slump records at job site shall not exceed the slump established for the mix design.
- C. Type Concrete. (28 day compressive strengths)
1. Footings, slab-on-grade, grade beams: 3,000 psi N.W.
2. Walls (Site, foundation, basement): 4,000 psi N.W.
3. Piers, columns: 4,000 psi, N.W.
4. All other structural concrete shown on these plans: 4,000 psi N.W. u.n.
5. All concrete exposed to the elements shall:
- a. be air-entrained 5% (+ 1%), and
- b. have crushed limestone aggregates.
6. Coarse aggregate shall meet ASTM Gradation #67.
7. All concrete in walls with only one plane of rebar shall have fiber reinforcement added to the concrete at the rate of 1.5 lbs/cyd.
- D. Formwork
1. See specs.
2. Keys indicated are to be 2 x 4 nominal continuous, u.n.
3. Camber: Provide camber to compensate for displacement of forms (see also specs) and to provide as-cast member cambers as noted on plans.
4. Rustication strips, chamfers, drips, mss, embeds, etc. See plans and/or architectural drawings.
- E. Reinforcement
1. Reinforcing Bars. Deformed, Grade 60, ASTM A615. Provide matching weldable rebar conforming to ASTM A706 where rebar welds are specified on contract documents.
- a. Fabrication: ACI 315 "Details and Detailing of Concrete Reinforcement or CRSI "Reinforcing Bar Detailing" (Manual of Standard Practice).
2. Welded Smooth Wire Fabric: ASTM A185
3. Welding: ACI 301, Section 5.3, AWS D1.4
4. Corner Bars. Provide corner bars same size and spacing as horizontal reinforcement at intersections of all walls, beams, footings, slabs and turn-downs. Tension Lap splice all corner bars, u.n.
5. Splices
- a. Continuous reinforcement bars shall be lapped 48 times the bar diameter, but not less than 1'-6" at all splices, u.n., including corner bars.
- b. Welded Wire Fabric shall be lapped 8".
6. Minimum cover in contact with ground: 3"
- a. Uniformed surface in contact with ground: 3"
- b. Formed surface in contact with ground or exposed to the elements: 2"
- c. Structural slabs and walls: 3/4"
- d. In all cases not less than the diameter of the bar.
7. Hooks indicated are ACI/CRSI standard 90 deg. or 180 deg. hooks. Bar lengths shown are out-to-out and do not include hook length. Embed hook 12" diameter (8" min.).
8. Provide support for miscellaneous top bars as follows:
- #4 carrying bars with chairs at 4'-0" o.c. max., OR #4 carrying bars with chairs at 3'-0" o.c. max.

9. Accessories: Provide protected bar supports Class 1. Maximum protection (CRSI Manual of Standard Practice).
10. Provide #3 spacer bars @ 8" o.c. in walls between mats of reinforcement in walls, with two or more mats of reinforcing.
11. Provide #4x2'-0" diagonal trim bar at each re-entrant corner of slab, and at each corner of each slab blockout, centered in the slab. Required in slab on grade and elevated slabs.
- F. Slab & Wall Openings: Coordinate with mechanical, plumbing and electrical trades. Field requirements not shown on structural plans must be approved by structural engineer. Additional reinforcement required for such openings shall be provided by the contractor at no additional cost to the Owner.
- Plumbing slots are to be filled with concrete to same thickness as slab or wall after piping has been installed.
- G. Embedded Pipes or Conduits:
- a. In slab on grade: Maximum diameter = 1/4 x slab or wall thickness, spaced minimum of 6 diameters on center, centered in the slab/wall.
- b. In elevated slabs: Maximum diameter = 1/4 x slab thickness, spaced minimum of 1'-6" on center, centered in the slab/wall.
- c. Cross over of conduits (one over another) shall not be permitted.
- d. Embedded pipes or conduits shall not be permitted in wall stems, beams or columns, unless noted otherwise on plans.
- H. Grout, epoxy mortar:
1. High strength non shrink grout: GR-621, 8000 psi.
2. Epoxy Mortar: Euclid EUCO #452 epoxy system with sand.
- I. Concrete Finishes: See specs. also.
1. Formed surfaces:
- a. Painted or exposed to view: rubbed, u.n. on plans.
- b. Covered or as noted on plans: as-cast.
2. Flatwork surfaces:
- a. Exposed to view: troweled
- b. Tiled or carpeted: troweled
- c. Stairs or ramps: non-slip
- d. Sidewalks, driveways: broomed or belted
- J. Slabs on Grade. Control joints shall be made by saw cut 3/16" wide x 7/4" deep or PWD joint strip (T-slab depth). Control joints shall be cut as soon as the concrete has set and before shrinkage has occurred on the surface. See plans for joint details.
- K. Thickened Slab on grade. Thickened slab shown on plans shall be cast monolithically with the rest of the slab on grade.
- L. Concrete over steel deck: Floor members are designed to be unshored unless otherwise noted. The weight of the wet concrete will result in deflections of at least Span/360 and as noted in the deck specifications. Overruns of concrete quantities are to be anticipated and included in the contractor's bid. Contractor shall coat concrete embedded items shown at the floor surface. Concrete floors utilizing unshored construction shall be screeded level.
- M. Shop Drawings: Submit rebar shop drawings. See specs.

- VI. STRUCTURAL STEEL: See specs
- A. Design, Fabrication, and Erection. AISC-360, Spec. for Structural Steel Buildings.
- B. Grade Steel.
1. Wide Flange sections: ASTM A992, Gr. 50.
2. Structural tube: ASTM A500, Gr. B, 46 ksi.
3. Structural Pipe: ASTM A53, Gr. B.
4. Misc. structural steel: ASTM A36.
- C. Connections. AISC Manual standard connections unless noted.
- Non composite beam connections that are not detailed on structural drawings shall be designed by steel supplier for 1.25 times the AISC maximum uniform service load capacity of the connected member (applies to both LRFD or ASD design).
1. All connections shall be bolted type, using 3/4" dia. A325N bolts and double angle framed connection, u.n.o.
2. High-strength bolts: ASTM A325 bearing type.
3. Standard AISC "Usual Gage" dimensions shall be used for locating holes for bolts, expansion anchors, etc. in all angles, beam flanges, etc. unless noted. All bolt holes shall be standard holes, u.n. Bolts shall be pretensioned per AISC Spec. Snug tight bolts shall not be permitted.
4. Welds: AWS D1.1 and AWS D1.8, Series E70XX. Provide 3/16" fillet welds, all around, u.n. Return welds around corners and ends.
5. Headed Studs: Nelson type H4L or S3L, ASTM A108 (Min. fy=50 ksi). All studs shall be stud welded per stud manufacturer's recommendations.
6. Rebar welded to steel shall be ASTM A706, grade 60, weldable type. Use E80 electrode. Weld size shall be capable of developing 1.2 times the tensile strength of the rebar. Comply with AWS D1.4.
- D. Tolerances. Such as:
1. All structural steel exposed to view in the finished building shall be considered AESS whether or not designated as such on plans. Such steel shall meet the tolerances for AESS, all welds shall be watertight and ground smooth.
2. Camber. Provide positive camber as noted on plans. Where no camber is noted, residual mill camber is to be upwards.
3. Floor and Roof Openings. Provide size and location of required openings with mechanical, plumbing, or other trades as necessary to assure correct framing and submit on shop drawings. Provide sub-support as required for deck closures typical, including cut columns. Coordinate with deck supplier.
4. Shop Drawings. Submit shop drawings. See specs. Submit calculations for connections, bracing, framing, stairs, etc. required to be designed by the fabricator for A/E review.
5. Composite and non composite floor members are designed to be unshored unless otherwise noted.
- I. Paint. Shop primed. Primer to be compatible with fireproofing material. All steel and steel accessories (including bolts, nuts and washers) exposed to elements shall be G60 galvanized.
- J. Joist erection stability. Holes in not cut holes in beam's tension flanges at a distance of 1/4 beam span (min. three feet) on each side of the point of maximum moment (i.e., at mid span on simple span beams, and at columns for cantilever or continuous beams). Provide additional miscellaneous steel plates, etc. as needed to provide holes for bolted connection of joist and bridging during erection.
- K. Double angle struts shall be connected to each other. The connector plate shall be 3/8" thick, u.n., at a maximum spacing of 0.3 times the length of the strut, u.n. The plates shall be welded to angles with 3/16" fillet weld all around, u.n.
- L. Roof top units: Provide shims and/or additional framing as required to provide a level surface to support the RTU. The contractor shall consider the deflection of the support framing under the Unit's weight. Shims may also be required to even out surface irregularities, due to bolt heads, cover plates, angles, etc.
- M. Provide L3x3x3/16 continuous at roof perimeter and interior edges, unless noted otherwise.
- N. Provide 3/16" bent plate with vertical leg equal to the slab depth and horizontal leg length as required for proper anchorage to the element which supports the bent plate continuous around the perimeter and at interior edges of the composite and non-composite slabs, unless noted otherwise.
- O. Do not cut holes in tube or pipe bracing, columns or beams, unless noted on structural drawings. Holes in tube or pipe steel shall not be permitted. Contractor shall coordinate and note on shop drawings conduit, pipes, etc. prior to submitting shop drawings for A/E review.
- P. Provide plate washers under anchor bolt nuts. Washers thickness shall be at least 1/3rd of the bolt diameter. The washer diameter shall be 1" greater than the hole in the base plate covered by the washer.
- Q. Provide weep holes in HSS members that are subject to weather or are galvanized.
- R. All cuts (cope, re-entrant corner, etc.) in the steel members shall have at least 1/2" radius, u.n. Torch cut edges must be ground smooth.

- OPEN-WEB STEEL JOISTS:
- A. Design, Fabrication, and Erection. Use, SJI Specs.
- B. Design roof joists to resist 20 psf net uplift; 30 psf net uplift in 12 feet wide strip along the perimeter of the roof, including cut eaves, overhangs, etc.
- C. Connection. Joists to be welded or bolted to supports as required by SJI. Extend bottom chord of joist to wall lines and connect after all dead loads are in place.
- D. Paint. One coat SSPC 15-68T. Paint to be compatible with fireproofing.
- E. Concentrated Loads. Attachment in such manner or at such location that local bending is not introduced into the chords except as noted.
- F. Provide standard SJI bridging, for erection stability as well as for uplift. Anchor bridging to beams, columns or walls, as recommended by SJI. Bridging and/or diagonal bracing to be collinear where exposed to view.
- G. Submit Shop Drawings. See specs.
- H. Joist erection stability. Provide extended seats as needed to make bolted connection of joists as required for erection stability. Coordinate with structural steel supplier.

- OR MECHANICAL ANCHORS IN MASONRY AND CONCRETE
- A. Epoxy Anchoring:
1. Hilti HIT-RE 500-SD Epoxy Adhesive for use in concrete.
2. Hilti HIT HY 70 Epoxy Adhesive for use in grout filled CMU.
- B. Mechanical anchor: Hilti Kwik Bolt or approved equal.
- C. All anchors shall be hot dipped galvanized.
- D. In concrete masonry, the anchors shall not be located in a head joint or within 2" of a head or T joint.
- E. Follow manufacturer's recommendations for installation procedures.
- F. Anchors in CMU walls must be centered in grouted cells and must have at least 12 inches of CMU wall all around the anchor fully grouted.
- G. Contractor's testing laboratory or approved inspector shall perform full inspections during the installation of anchors as outlined by the manufacturer in its ICC-ES Evaluation Reports.
- H. Concrete and masonry substrates receiving epoxy anchors shall be dry. All holes shall be also be dry prior to injecting epoxy into the hole.
- I. All fasteners in contact with preservative treated lumber or exposed to moisture shall be hot dipped galvanized.
- J. Contractor shall locate (with non destructive means only) reinforcement in the existing concrete or masonry prior to drilling. Do not cut or damage reinforcement. Obtain written A/E approval prior to cutting rebar.

- IX. STEEL NON COMPOSITE FLOOR DECK (See Specs.)
- A. Design: SDI, Specifications and Commentary.
- B. Material: ASI, Specification for the Design of Cold Formed Steel Structural members, Section 1.2 Material. Minimum yield point, Fy=80 ksi. Minimum thickness and depth = see plan.
- C. ASTM A563 grade 80, Galvanized.
1. Protective Coating: ASTM A924, G60 Galvanized.
- C. Installation: SDI Specifications, Section 4 (see specs). Deck attachments shall be in accordance with SDI specs unless noted and shall be adequately shown on deck shop drawings for field installation. Attach to framing with 5/8" dia. puddle welds with 12" o/c (include 12" o/c max. at all end and intermediate supports. Attach sideclips with #10 screws at 12" o/c. Attach deck to framing at perimeter at 6" o/c. Provide a minimum 1"-12" sideclap.
- D. Concrete. Refer to plan above. Chloride salts or admixtures containing chloride salts shall not be used.
- E. Deck Supplier to provide all closures, etc. necessary to detail concrete. Coordinate Sub-support with steel fabricator.
- F. Four joints: Concrete pour joints shall occur in the middle third region of the slab between beams, and girders. Joints shall not be allowed over or near any beams or girders, except as noted above.
- H. Shop Drawings: Submit Shop Drawings.
- X. PRE-ENGINEERED PRE-FABRICATED LIGHT GAGE STEEL TRUSSES: See Spec.
1. Design. ASI "Specifications for the Design of Cold Formed Steel Structural Members". Top chord thickness shall not be less than 18 gauge.
- a. Design Loads, unless noted otherwise on plans:
- Roof Live Load = 20 psf
- DL Top chord = 10 psf
- DL Bottom chord = 10 psf
- b. Deflections. Roof truss (Live load)/L/360
- Total DL+LL = L/240 (for roof and floor)
- c. Roof truss uplift: 20 psf net; 30 psf net at eaves, overhangs and corners, u.n.o. on plans.
- d. Truss overhang/cantilever shall be designed to support minimum 500 lbs concentrated live load at the cantilever end.
- e. Scissor trusses shall be designed assuming one of the supports to be a roller, such that lateral displacement is allowed.
- f. Where parallel chord trusses are used for roof framing, the trusses shall be designed to support ponding related stress. Refer to ASCE Standard 16-95 (Appendix A.3).
- g. See plans for additional loads from snow, ruf, etc.
2. Design the truss to resist all loads due to supporting structure anchorage and connections shall be designed by the truss supplier.
3. Shop Drawings. Submit shop drawings and calculations, signed and sealed by a Registered Engineer, showing all anchorage and connection details, hardware, truss design, layout, truss members, bridging, bracing and installation details.
4. Truss layout shown on structural drawings shall not be revised in a way that changes the load path to the foundations.
5. All truss layout shown on structural drawings shall be coordinated with the design engineer.
6. Provide camber at mid span equal to 80% of dead load for parallel chord trussing.
7. Field verify all as-built support conditions for position, levelness, slope, etc. prior to fabrication. Adjust fabrication lengths, thickness, etc. to fit existing as-built support structure.

- XII. TEMPORARY BRACING OF STRUCTURE
- A. Contractor shall provide temporary bracing as required until all lateral force resisting elements are in place (such as roof joist erection stability, bracing, shearwalls, etc.)
- B. Contractor shall provide all erection stability bracing, bridging, blocking, etc. as required during construction.
- C. Contractor shall be solely responsible for the construction means and methods required to safely achieve the conditions depicted in the contract documents. Where construction sequence is noted on the documents, they should be treated as recommendations and are not mandatory. However, any variation in the recommended sequence should be submitted to A/E prior to construction.
- D. All excavations shall be shored. Unshored excavations shall meet the geotechnical engineer's recommendations.
- XIII. EXISTING CONSTRUCTION/UTILITIES
- Contractor shall field verify existing conditions noted on these plans (dimensions, elevations, construction details, etc.) and to coordinate same in all affected shop drawings prior to submittal.
- B. Report any existing conditions not as indicated or not shown on the contract documents to A/E prior to fabrication and/or construction.
- C. Refer to reports/surveys by others for subsurface and above ground existing conditions as necessary.
- D. Contractor shall locate (with non destructive means only) reinforcement in the existing concrete or masonry prior to drilling. Do not cut or damage reinforcement. Obtain written A/E approval prior to cutting rebar.
- E. New roof top units on an existing roof system may cause significant deflection which may lead to ponding of rainwater on the roof. The contractor shall verify that there is a positive drainage away from the units to a drainage system after the units are installed. Contractor shall modify existing roofing system as needed to maintain slope to a drain.
- F. Protect existing utilities, roofing, equipment and structure from damage during construction.
- G. Contractor shall field verify soundness of existing structural members (joists, beams, columns, etc.) and their connections prior to construction. Any member or connection if found to be defective, damaged or distressed shall be tagged and brought to the Engineer's notice prior to construction.
- H. All existing rebar and structural steel surface shall be cleaned to meet SSPC SP-6 standard prior to attachment to new construction.
- I. All existing concrete or masonry shall be cleaned to meet SSPC SP-13 standard prior to applying bonding agent and placement of adjacent concrete.

- XIII. EPOXY OR MECHANICAL ANCHORS IN MASONRY AND CONCRETE
- A. Epoxy Anchoring:
1. Hilti HIT-RE 500-SD Epoxy Adhesive for use in concrete.
2. Hilti HIT HY 70 Epoxy Adhesive for use in grout filled CMU.
- B. Mechanical anchor: Hilti Kwik Bolt or approved equal.
- C. All anchors shall be hot dipped galvanized.
- D. In concrete masonry, the anchors shall not be located in a head joint or within 2" of a head or T joint.
- E. Follow manufacturer's recommendations for installation procedures.
- F. Anchors in CMU walls must be centered in grouted cells and must have at least 12 inches of CMU wall all around the anchor fully grouted.
- G. Contractor's testing laboratory or approved inspector shall perform full inspections during the installation of anchors as outlined by the manufacturer in its ICC-ES Evaluation Reports.
- H. Concrete and masonry substrates receiving epoxy anchors shall be dry. All holes shall be also be dry prior to injecting epoxy into the hole.
- I. All fasteners in contact with preservative treated lumber or exposed to moisture shall be hot dipped galvanized.
- J. Contractor shall locate (with non destructive means only) reinforcement in the existing concrete or masonry prior to drilling. Do not cut or damage reinforcement. Obtain written A/E approval prior to cutting rebar.

- XIV. SUPPLEMENTARY NOTES
- A. For connections, see details. If not shown or noted, minimum connections to be included in cost shall be two 3/4" dia. bolts or 3/16" fillet weld 4" long using 3/8" connection material and detailed to minimize bending in connection. Proceed after approved or clarified on the shop drawing submittals.
- B. Unless otherwise noted, details and sections on structural drawings are typical and indicated by refs, references or titles. All details shown shall be imported into the project at all appropriate locations, whether specifically indicated or not. Typical details may not be referenced on the documents, but shall apply at all locations, unless noted otherwise. Where no detail cuts are shown construction shall conform to similar work shown elsewhere in the project documents. For bidding purposes, where any shown member, rebar or structural element is not sized on the documents, the largest similar member, rebar or structural element used in the project shall be utilized.
- C. For clarity, all openings may not be shown on drawings. See also architectural, mechanical, electrical and plumbing plans. All openings and penetrations shall be located and verified by all trades from drawings made by them. Contractor shall not proceed with any work shown on drawings if in conflict until receiving clarification from the architect. For framing at openings, see typical structural details.
- D. All construction meeting or crossing expansion or shrinkage control joints in framed floors, roof or slab on grade must have provisions to accommodate the movement or must be delayed until the joint has stopped growing or is closed.
- E. All above grade supporting structure shall be in place before existing and new buildings shall be considered seismic separation joints. The joint cover shall be capable of accommodating the maximum lateral movement caused by the sun above buildings movement toward and away from each other simultaneously.
- XV. SPECIAL INSPECTIONS
- A. Special structural inspection & verification by certified Special Inspector satisfactory to the building official is required in conformance with IBC code sections 1703, 1704, 1705 and 1707.
- B. The special inspector shall send copies of all structural inspection reports directly to the contractor, A/E and the building official. Any construction which fails to meet the contract documents shall immediately be brought to the A/E's attention. Special inspection requirements apply also to all construction.
- C. Material testing does not constitute special inspection. Such testing shall be done by an independent testing lab.
- D. ASTM Chapter 17 for details of Special inspection and verification requirements.
- E. The following construction shall be subject to Special Inspection and verification:
1. Soils (Section 1705.6)
2. Concrete (Section 1705.3)
3. Steel (including cold formed steel)(Section 1705.2, 1705.11)
4. Masonry (Section 1705.4)
5. Wood (Section 1705.5, 1705.11)
- F. The contractor shall notify the Special Inspector at least 24 hours prior to the work that is ready for inspection. Contractor shall also provide to the Inspector the necessary documents (approved submittals, etc.) and safe access to the work to be inspected.
- XVI. WOOD DECK:
- A. PLYWOOD DECK:
1. Design. NFPA and Supplements.
2. Plywood
- a. Roof deck: 19/32" APA Rated sheathing
- b. 32/16 span rating, Exterior grade, U.N.
3. Connections: See plan.
4. Provide ptyclips at all edges of plywood at no more than 6" o/c.
- XVII. LIGHT GAGE STEEL WALL FRAMING
- A. Light gage steel wall vendor to design all exterior and interior (load bearing and non load bearing) studs, tracks, slip tracks, headers, jambs and sills, including their connections to each other and to the supporting structure. Deflections shall meet the requirements of the latest edition fo the build code. Exterior stud walls shall be designed for wind pressure from a 120 mph wind using ASCE-7. Studs backing brick veneer shall be designed for maximum deflection of L/600.
- B. Stud vendor to submit detailed shop drawings indicating stud size and spacing, track sizes, gage, web stiffeners, blocking, bridging, etc. Bracing shall include number and location of fasteners to be used in construction. Shop drawings shall be signed and sealed by the design engineer registered in the State the project is located in.
- XVIII. LIGHT GAGE ZEE PURLINS
- A. Light gage steel zee purlin vendor to design all roof purlins and their connections to the supporting structure, including laps, bridging, bracing, etc.
- B. Maximum total deflections shall not exceed span/240 for total load and span/360 for live load. See plan notes for roof loads.
- C. Purlin designer/vendor to submit detailed shop drawings indicating size,spacing, gage, connection details, all loads including concentrated loads, deflection requirements, etc.
- D. All purlin connections and locations shall be coordinated with structural steel fabricator prior to shop drawings submittal.
- E. General contractor to provide purlin designer with locations and operating weights of all mechanical equipment, and architectural components to be supported by the purlins over 75 lbs in weight.
- F. Shop drawings shall be signed and sealed by the design engineer registered in the State the project is located in.

FOOTING SCHEDULE

MARK	SIZE	DEPTH	TOTAL REINFORCEMENT FOR EACH FOOTING			
			NO.	SIZE	LENGTH	REMARKS
F30	2'-6"x2'-6"	1'-0"	6	4	2'-0"	1/2 EA. WAY BOT.
F42	3'-6"x3'-6"	1'-0"	10	4	3'-0"	1/2 EA. WAY BOT.
F48	4'-0"x4'-0"	1'-0"	8	5	3'-6"	1/2 EA. WAY BOT.
F54	4'-6"x4'-6"	1'-0"	8	5	4'-0"	1/2 EA. WAY BOT.
F60	5'-0"x5'-0"	1'-0"	10	5	4'-6"	1/2 EA. WAY BOT.
F72	6'-0"x6'-0"	1'-0"	12	5	5'-6"	1/2 EA. WAY BOT.
F72A	6'-0"x6'-0"	2'-0"	12	5	5'-6"	1/2 EA. WAY TOP
F102A	8'-6"x8'-6"	2'-0"	16	6	8'-0"	1/2 EA. WAY BOT.
F102A	8'-6"x8'-6"	2'-0"	16	5	8'-0"	1/2 EA. WAY TOP
F114A	9'-6"x9'-6"	2'-0"	18	5	9'-0"	1/2 EA. WAY TOP
F120A	10'-0"x10'-0"	2'-0"	16	7	9'-6"	1/2 EA. WAY BOT.
F120A	10'-0"x10'-0"	2'-0"	20	5	9'-6"	1/2 EA. WAY TOP
F126A	10'-6"x10'-6"	2'-0"	18	7	10'-0"	1/2 EA. WAY BOT.
F126A	10'-6"x10'-6"	2'-0"	22	5	10'-0"	1/2 EA. WAY TOP
F132A	11'-0"x11'-0"	2'-0"	20	7	10'-6"	1/2 EA. WAY BOT.
F132A	11'-0"x11'-0"	2'-0"	22	5	10'-6"	1/2 EA. WAY TOP
F138A	11'-6"x11'-6"	2'-0"	24	7	11'-0"	1/2 EA. WAY BOT.
F138A	11'-6"x11'-6"	2'-0"	24	5	11'-0"	1/2 EA. WAY TOP
W18	1'-6" CONT.	1'-0"	2	#5	CONT.	BOTTOM
W24	2'-0" CONT.	1'-0"	3	#4	CONT.	@48" BOTTOM TRANS.
W24	2'-0" CONT.	1'-0"	3	#5	CONT.	BOTTOM
W48	4'-0" CONT.	1'-0"	5	#4	CONT.	@24" BOTTOM TRANS.
W48	4'-0" CONT.	1'-0"	5	#5	CONT.	BOTTOM
W48	4'-0" CONT.	1'-0"	5	#5	CONT.	@10" BOTTOM TRANS.

NOTES:

1. F36, ETC. DENOTE ISOLATED COLUMN FOOTING; W24, ETC. DENOTE CONTINUOUS WALL FOOTING.

2. ADDITIONAL ANCHORAGE REBAR IS REQUIRED IN FOOTINGS. SEE 5/S11.1.

TOM ROBISON & ASSOCIATES, INC.
STRUCTURAL ENGINEERS
1715 KIRBY PARKWAY - SUITE 201
MEMPHIS, TENNESSEE 38120
901.754.4832
sarfrax_k@bellsouth.net

PROJECT NO. 160103
DATE: 05-21-18
DRAWN BY: RC

REVISION:
DATE:

NOTICE:
THIS DRAWING IS THE PROPERTY OF MATT SILAS ARCHITECT. IT IS TO BE USED SOLELY FOR THIS PROJECT.

COPYRIGHT © 2018

MSA
MATT SILAS ARCHITECT
212 East Washington Ave.
Jonesboro, Arkansas 72401
Tel: (870) 268-0500 Fax: (870) 268-0501
Website: mattsilasarchitect.com Email: mattsilas@sbcbjohal.net

CENTRAL BAPTIST
CHURCH
ADMINISTRATION
WING ADDITION

3707 HARRISBURG ROAD
JONESBORO, ARKANSAS
72404

centralbaptist

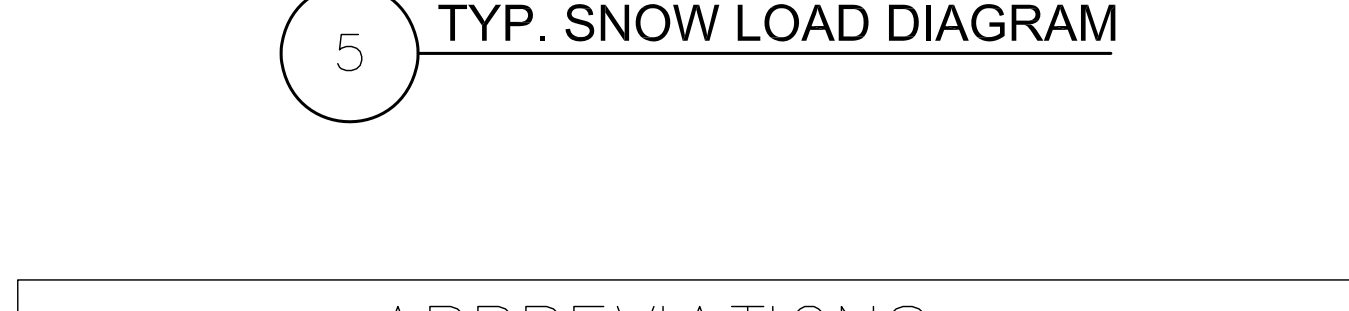
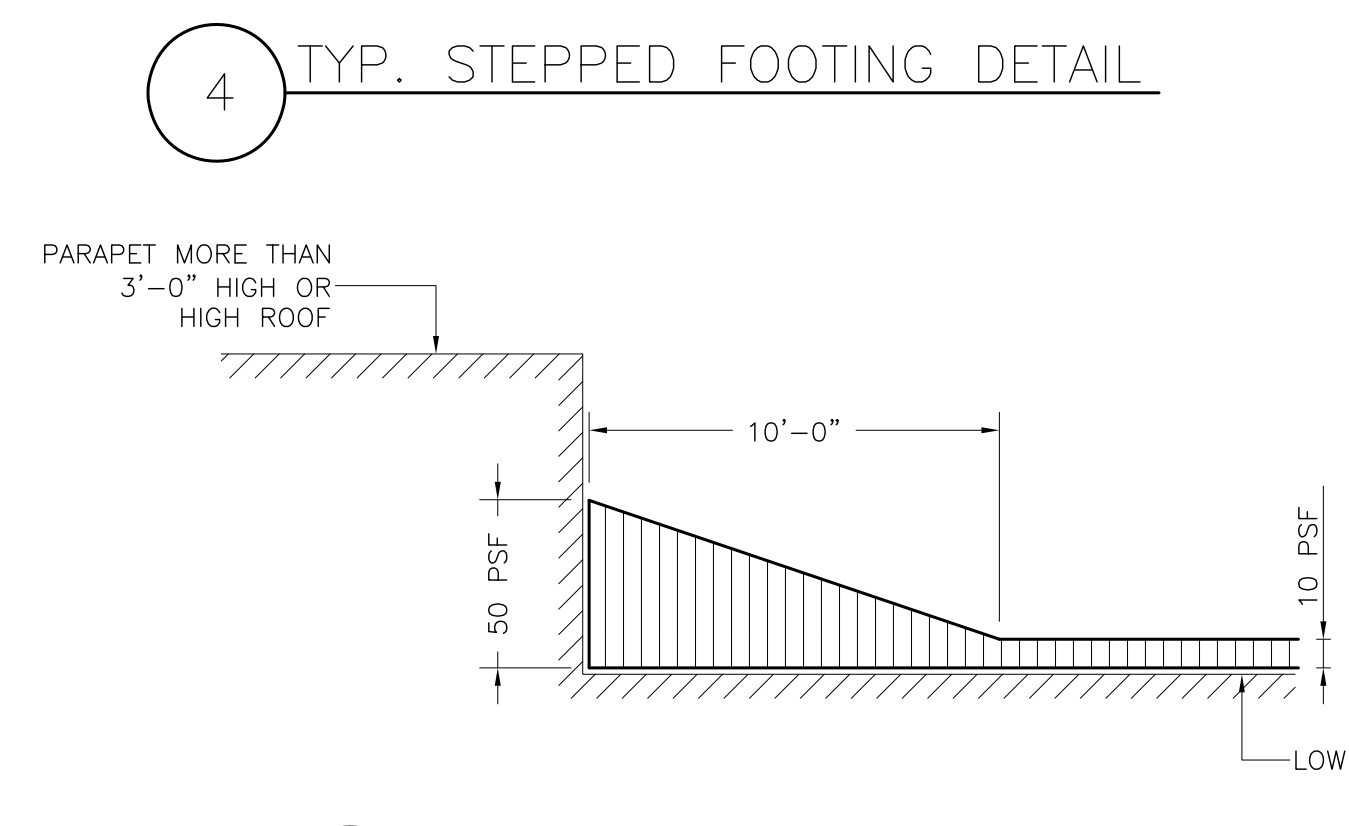
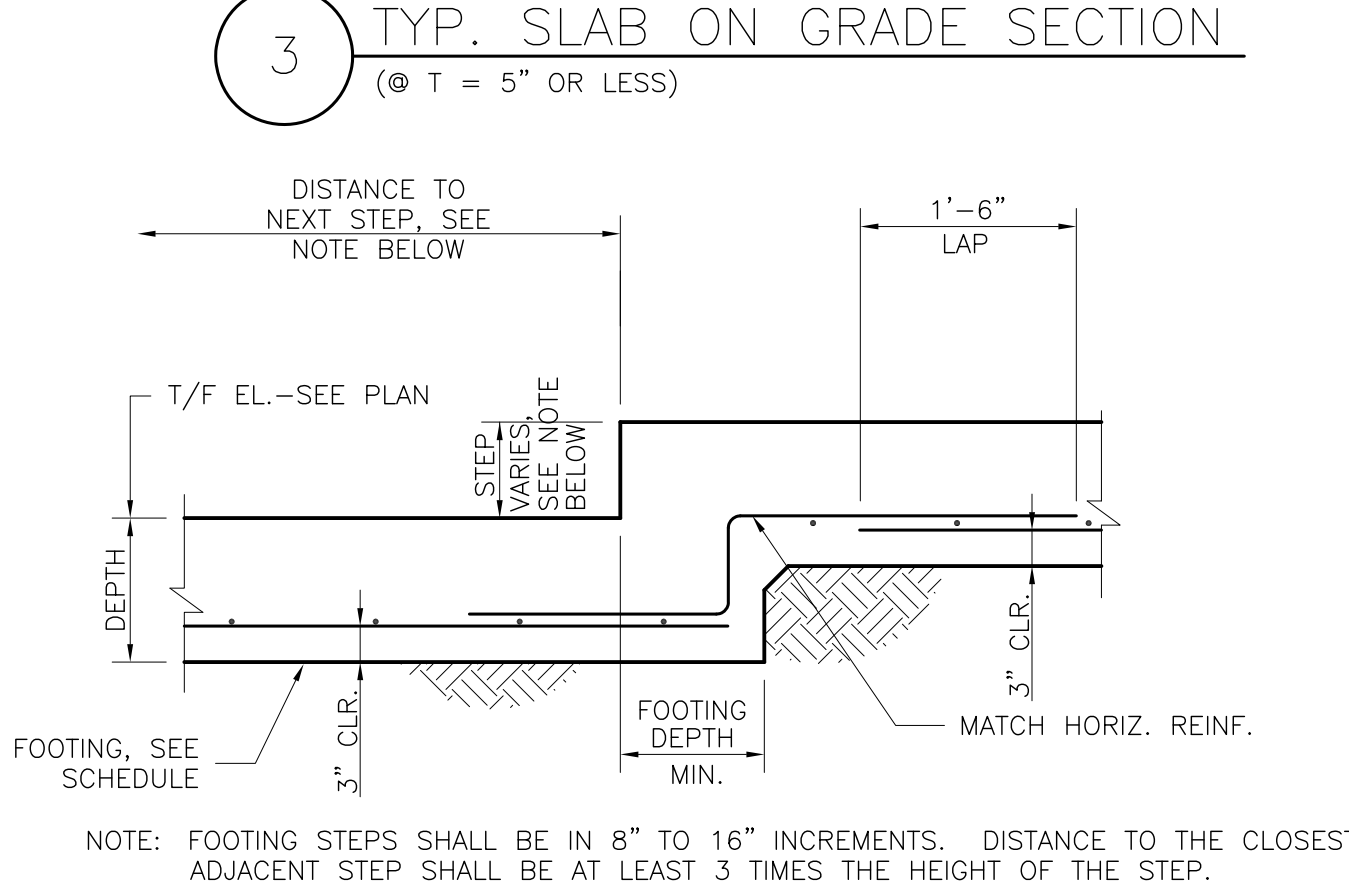
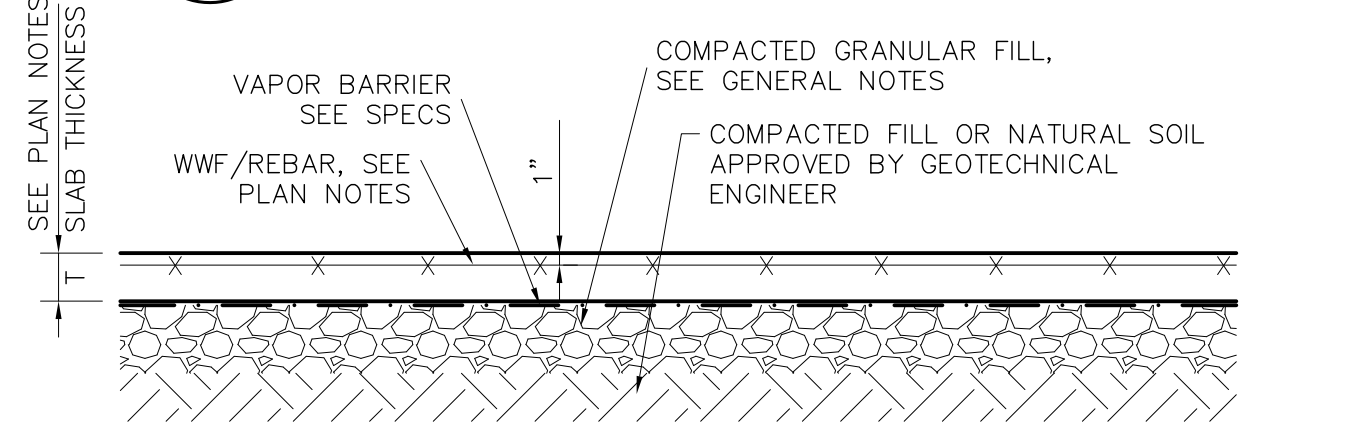
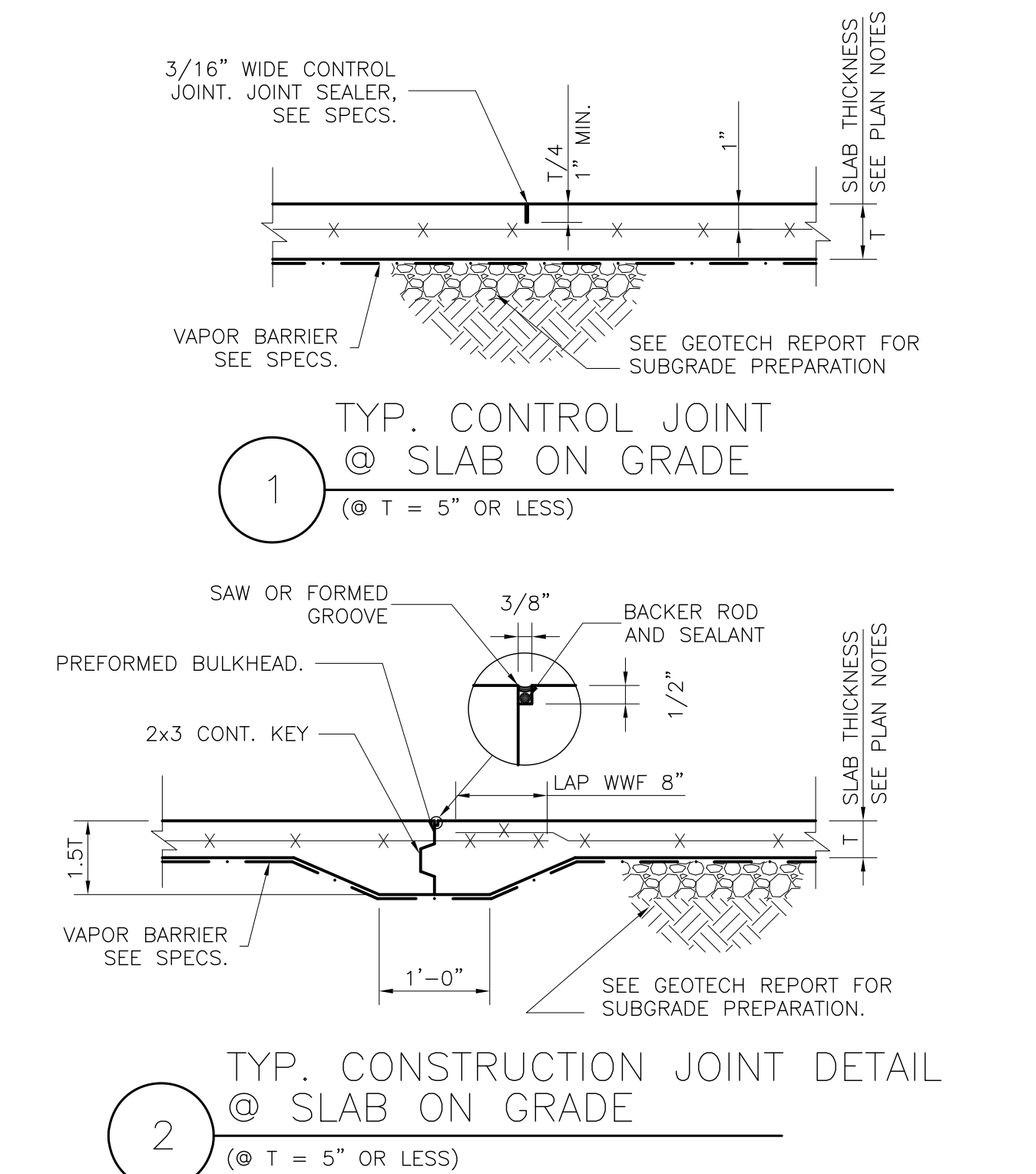
SEAL
CERTIFICATE OF AUTHORITY
TOM ROBISON
& ASSOCIATES, INC.
No. 1190
ARKANSAS

SEAL
STATE OF ARKANSAS
LICENSED PROFESSIONAL ENGINEER
No. 1485
EXPIRATION DATE 12/31/18

GENERAL NOTES
SCHEDULES &
TYPICAL DETAILS

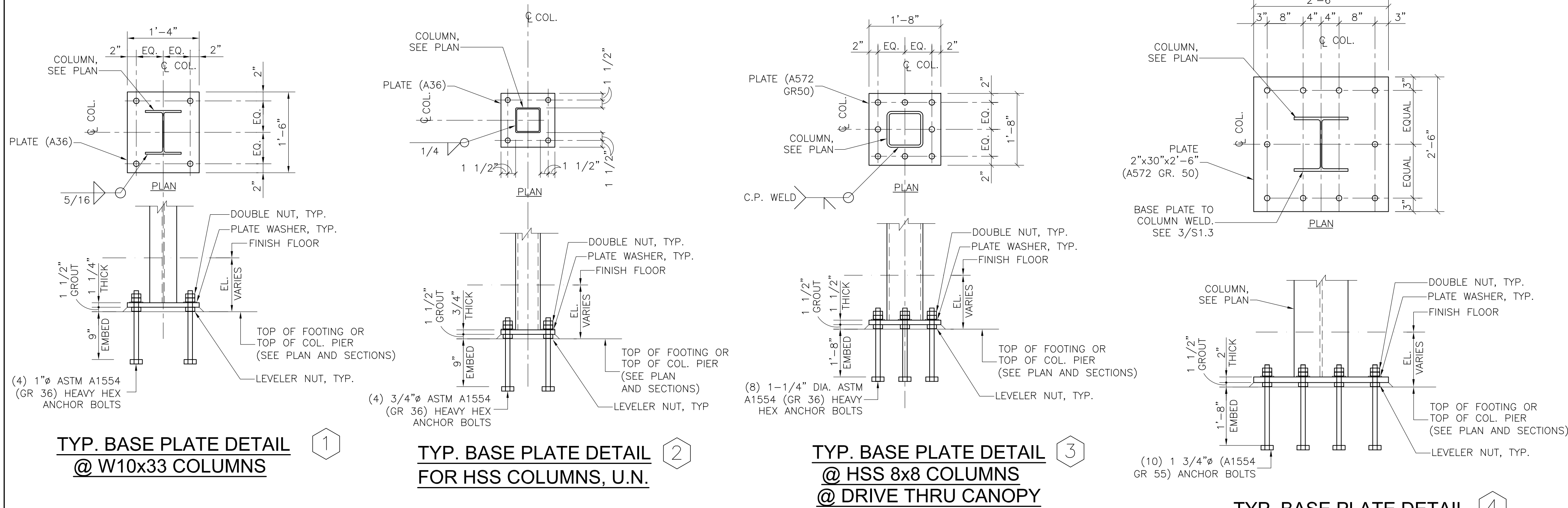
S1.0

SHEET NO.

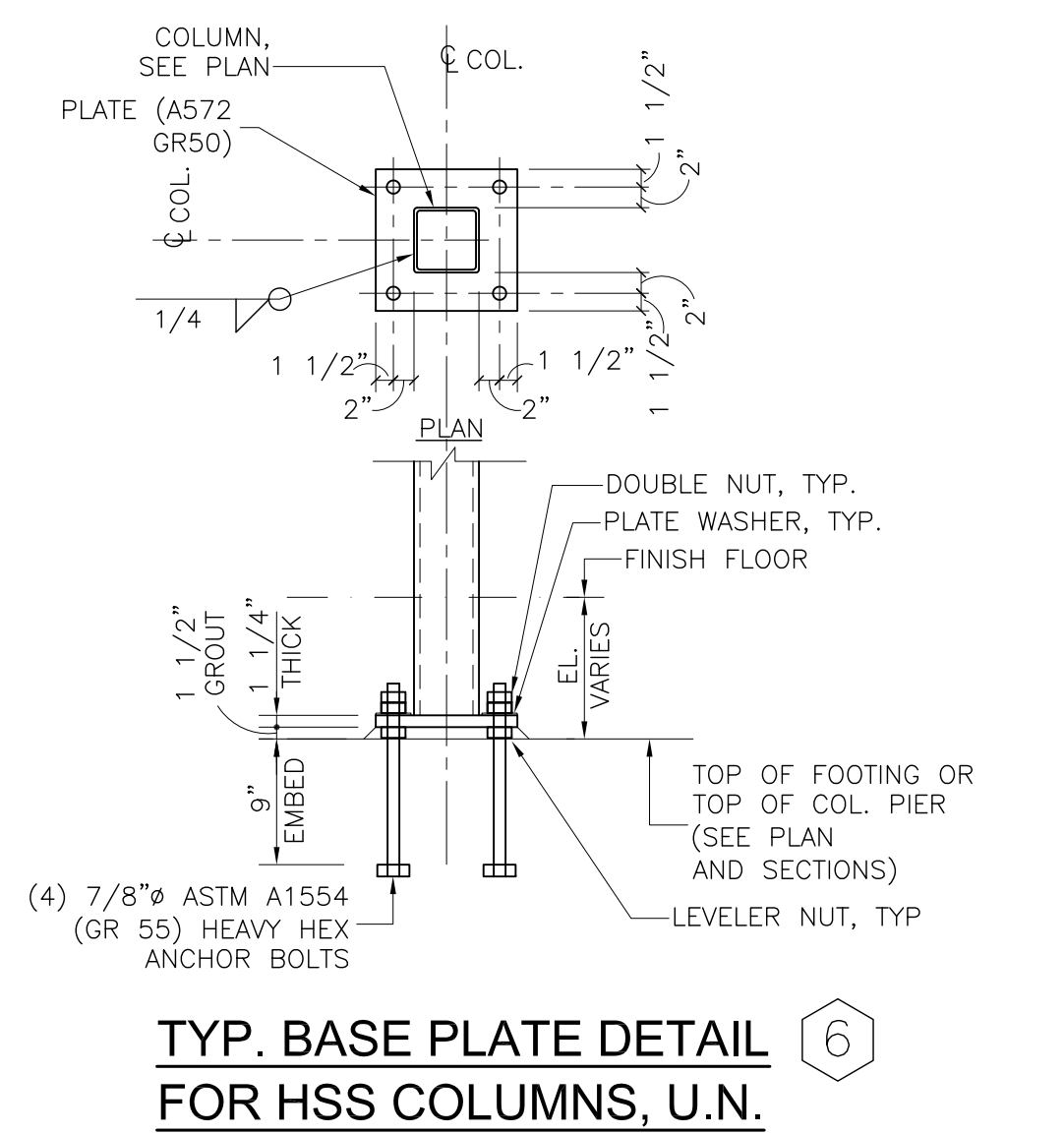


ABBREVIATIONS			
A.B.	ANCHOR BOLT	JT.	JOINT
A/E	ARCHITECT/ENGINEER	LDH	LONG DIMENSION HORIZONTAL
A.O.F.	ABOVE FINISHED FLOOR	LLV	LONG LEG/SIDE VERTICAL
ANCH.	ANCHOR	L.W.	LIGHTWEIGHT CONCRETE
ARCH.	ARCHITECTURAL	MAX.	MAXIMUM
ATR	ALL THREAD ROD (A36 GR.)	(N)	NEW
BASE	BASE	N.T.S.	NOT TO SCALE
BM	BEAM	O.C.	ON CENTER
BRG	BEARING	O.H.	OPPOSITE HAND
CMU	CONCRETE MASONRY UNIT	W.O.	WALL WEIGHT CONCRETE
BOTT	BOTTOM	OSHA	O.S.H.A. REGULATIONS
CFV	CONTRACTOR FIELD VERIFY	PL	PLATE
CLR. CVR.	CLEAR COVER	REF.	REINFORCING
CONC.	CONCRETE	REM.	REMAINDER
CONV.	CONNECTION	REQ'D	REQUIRED
CONST.	CONSTRUCTION	RTU	ROUGH/MASONRY OPENING
COORD.	COORDINATE	S.J.	SAW-CUT JOINT
COR	CORNER	SIM.	SIMILAR
DEG.	DEGREE	SPEC.	SPECIFICATIONS
DWS.	DRAWINGS	SQ.	SQUARE
E.	EXISTING	T.O.F.	TOP OF FOOTING
E.F.	EACH FACE	T&B	TOP AND BOTTOM
E.W.	EACH WAY	TOP	TOP
EACH	EACH	TYP.	TYPICAL
EL.	ELEVATION	U.N.	UNLESS NOTED
EOC	EDGE OF CONCRETE	VERT.	VERTICAL

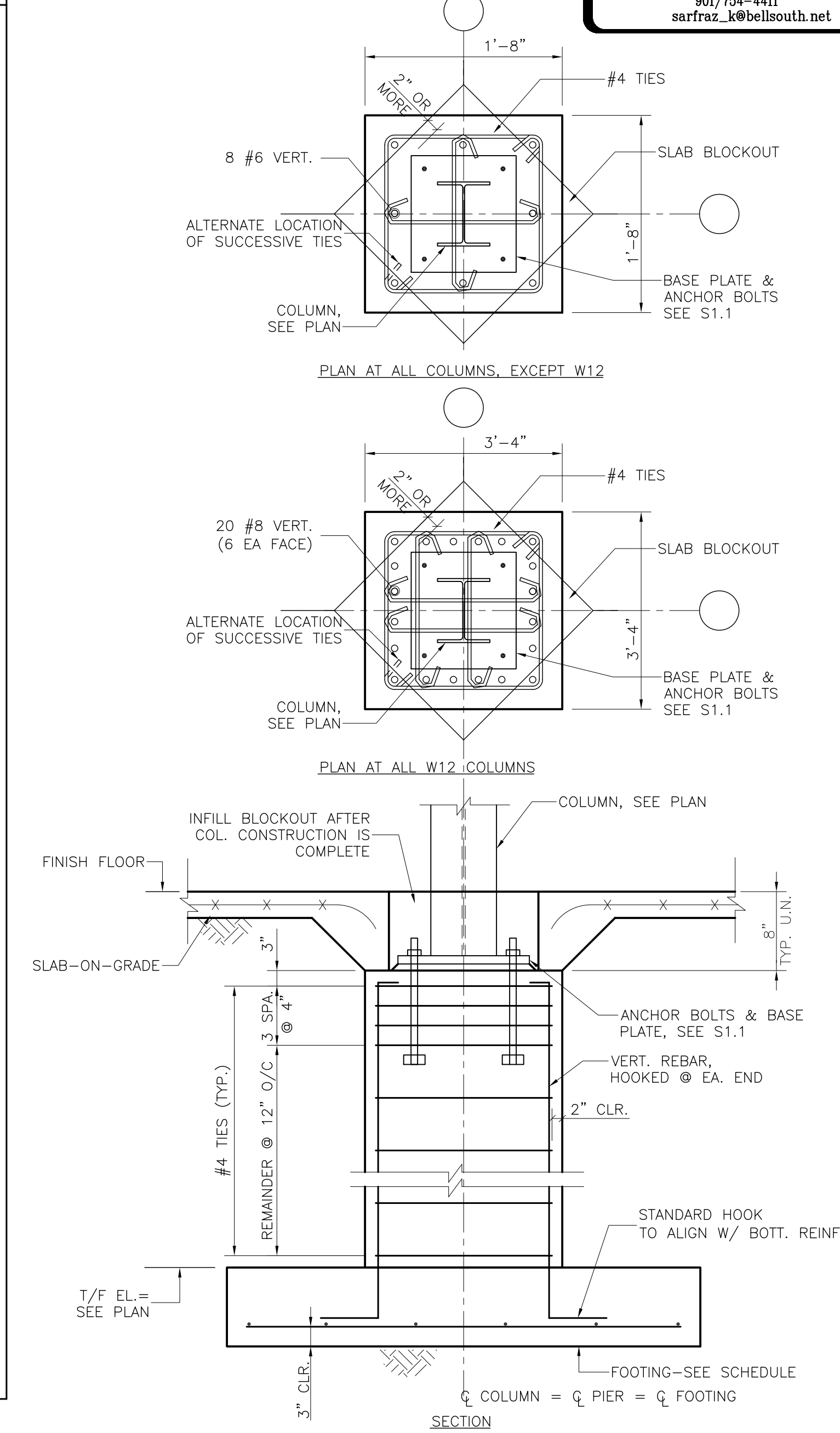
TYP. BASE PLATE DETAILS



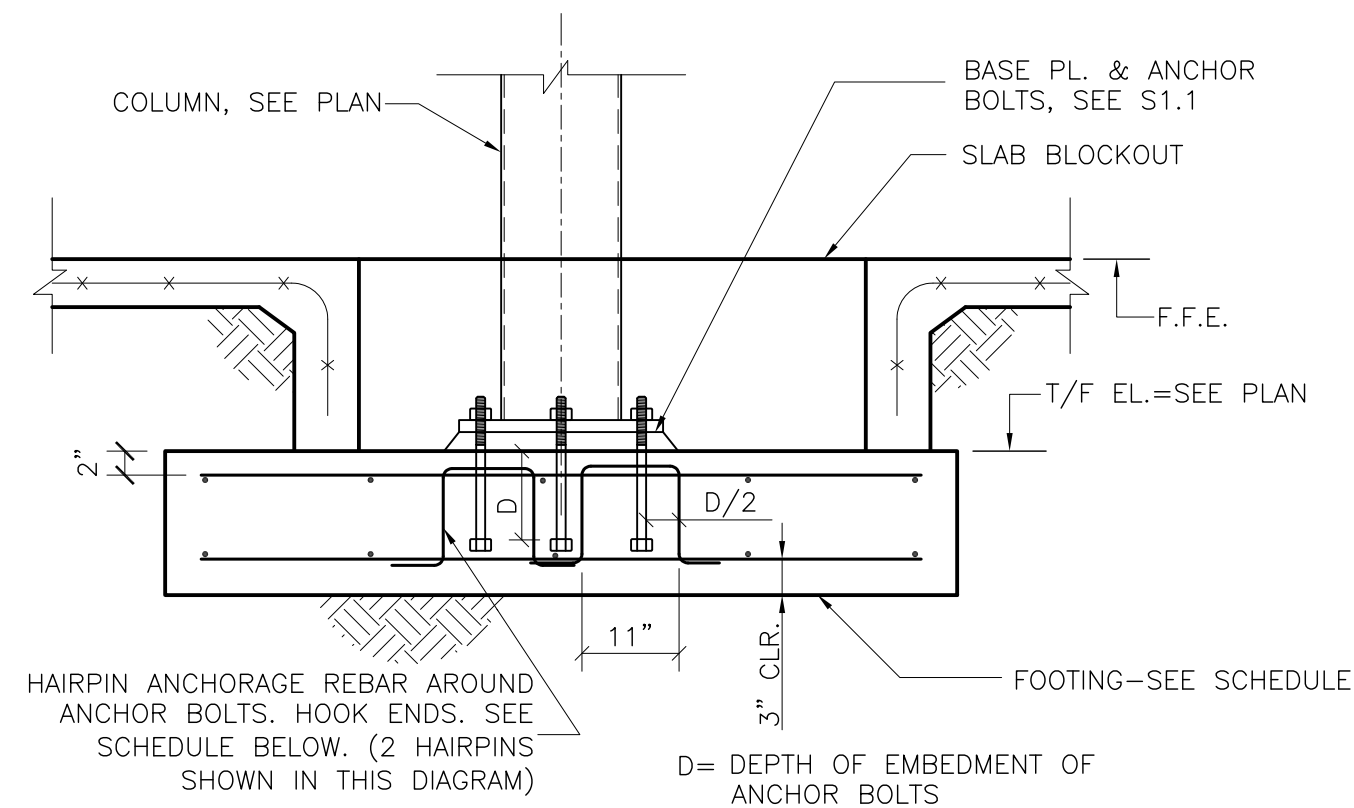
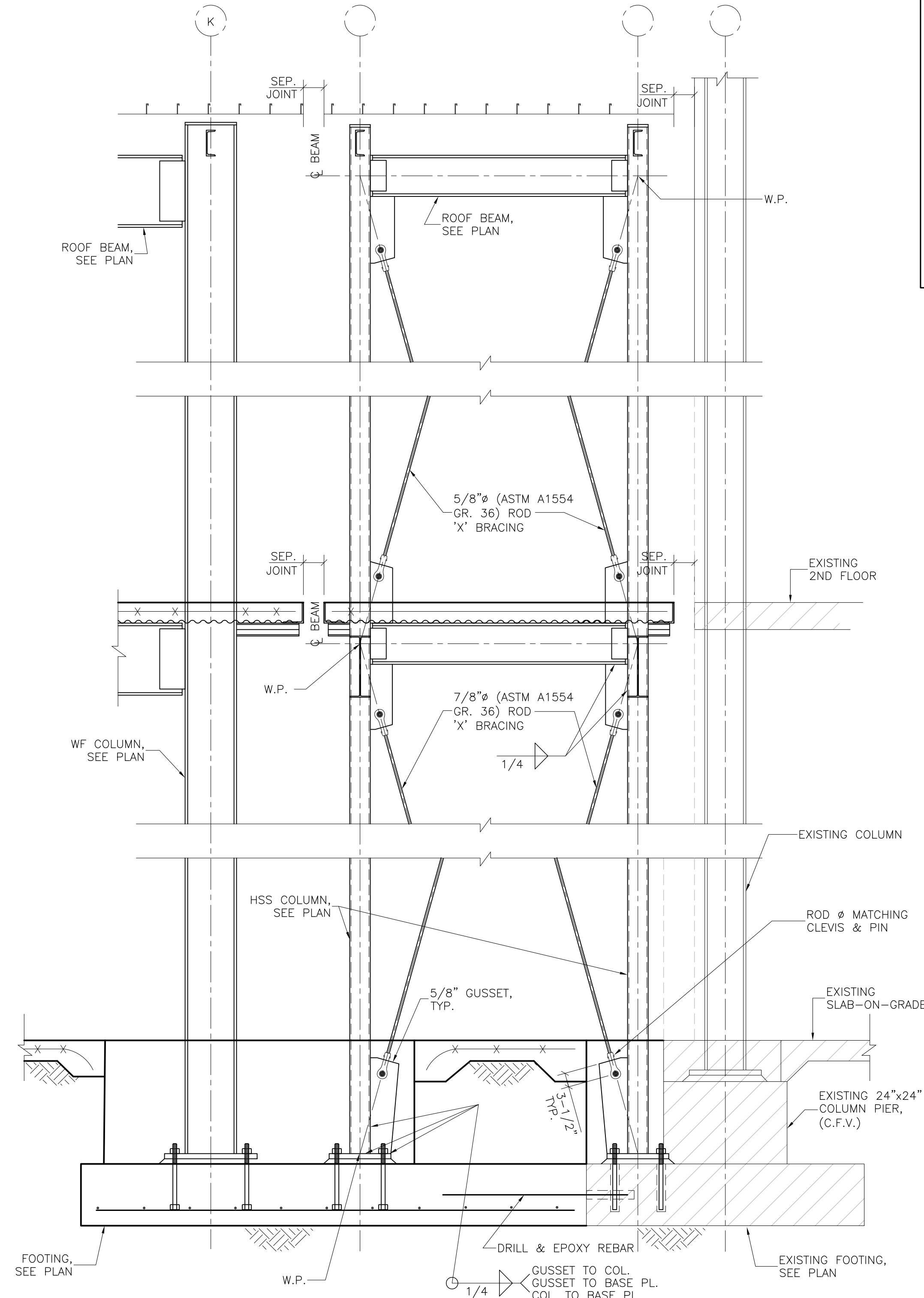
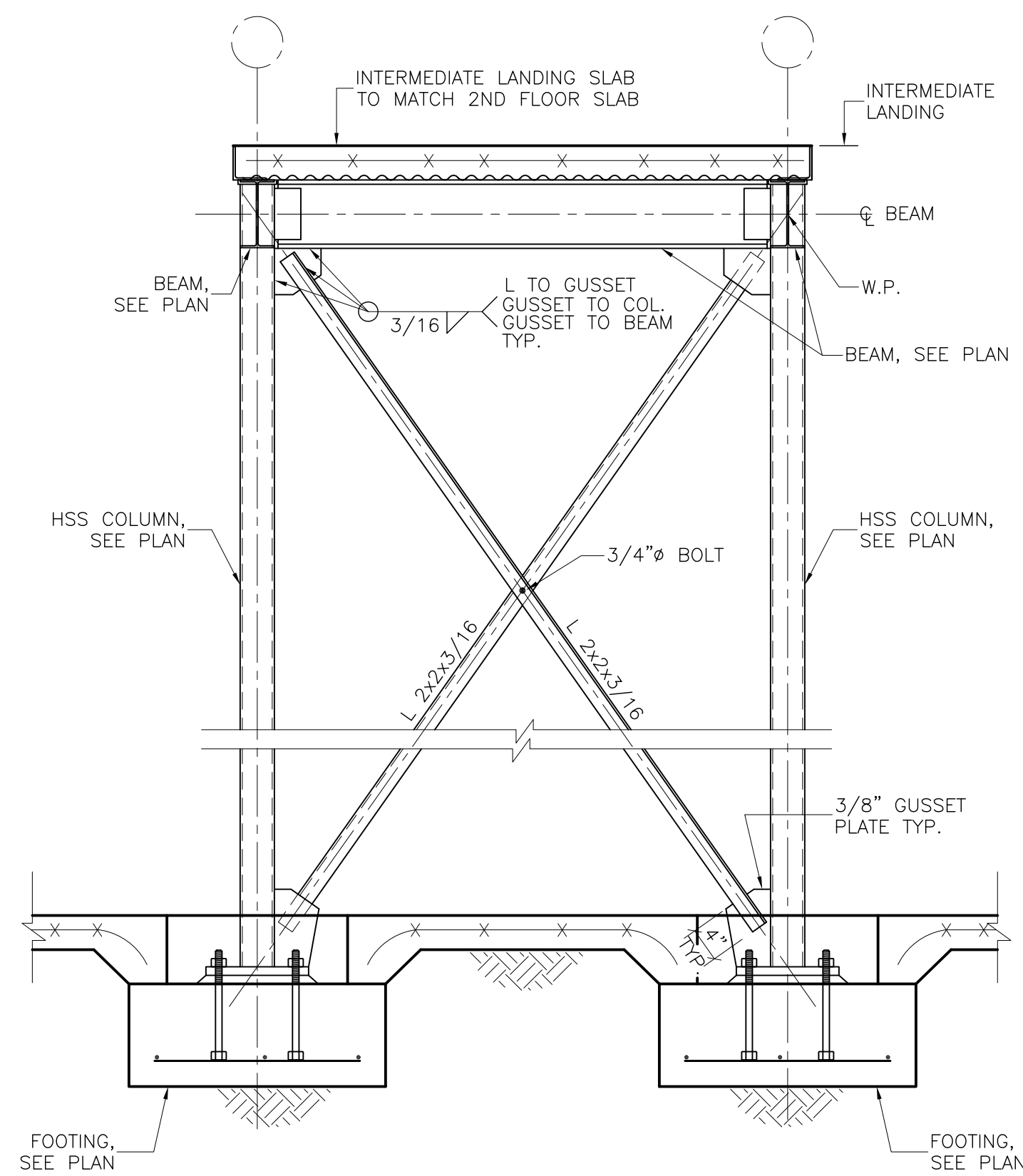
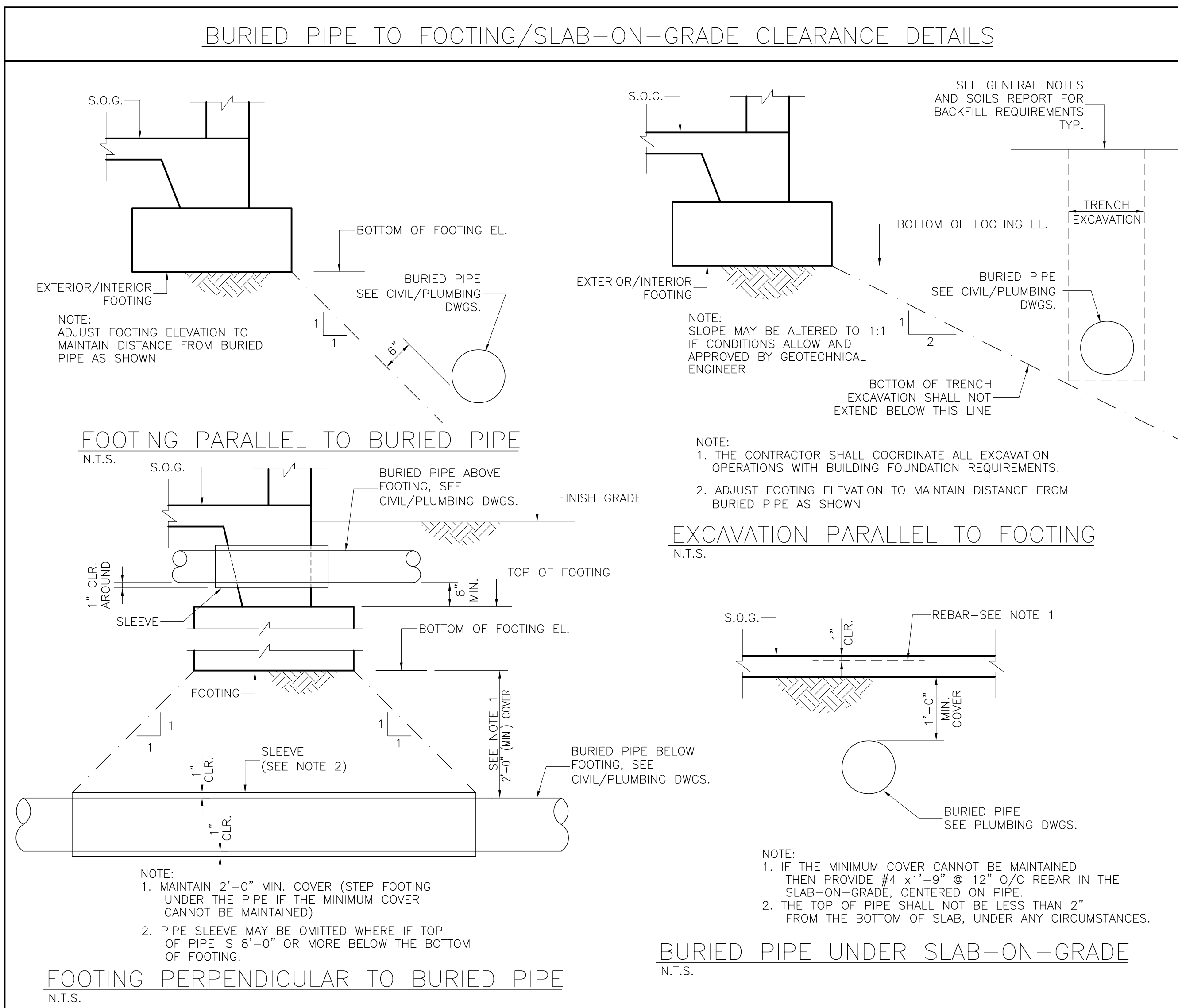
TYP. BASE PLATE DETAIL @ COLUMNS ON EXIST. FOOTINGS



CONCRETE PIER



NOT USED



LOCATION	REBAR DIA.	NUMBER OF HAIRPINS PER SIDE OF BASE PLATE
CANOPY (6 COLUMNS)	#5	2 EACH SIDE = (4 X 2 = 8 TOTAL PER COL)
ALL W12x136 COLUMNS	#6	3 EACH SIDE = (4 X 3 = 12 TOTAL PER COL)

TOM ROBISON & ASSOCIATES, INC.
STRUCTURAL ENGINEERS
1716 KIRBY PARKWAY - SUITE 201
MEMPHIS, TENNESSEE 38120
901/754-4411
sarfrax_k@bellsouth.net

PROJECT NO. 160103
DATE: 05-21-16
DRAWN BY: RC

REVISION:
DATE:

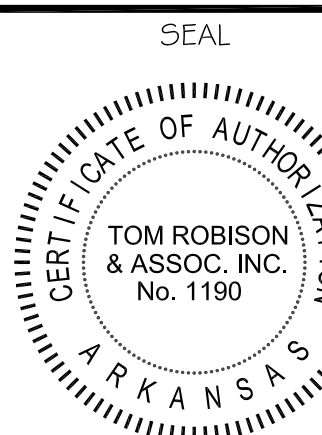
NOTICE
THIS DRAWING IS THE PROPERTY OF MATT SILAS ARCHITECT. IT IS TO BE USED SOLELY FOR THIS PROJECT.
COPYRIGHT © 2018

MSA
MATT SILAS ARCHITECT
Jonesboro, Arkansas 72401
212 East Washington Ave.
Tel: (870) 268-0500 Fax: (870) 268-0501
Website: mattsilasarchitect.com Email: mattsilas@bellsouth.net

CENTRAL BAPTIST CHURCH
ADMINISTRATION WING ADDITION

3707 HARRISBURG ROAD
JONESBORO, ARKANSAS 72404

centralbaptist

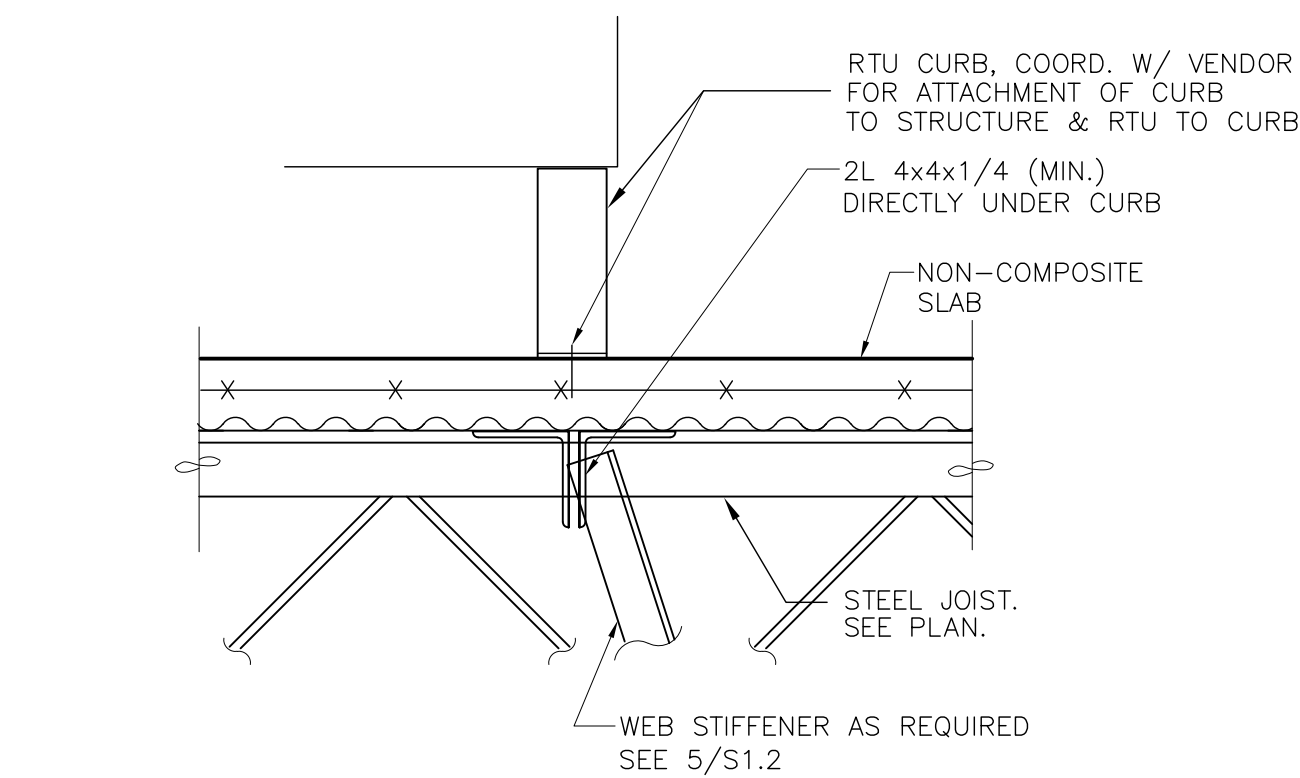


TYPICAL DETAILS

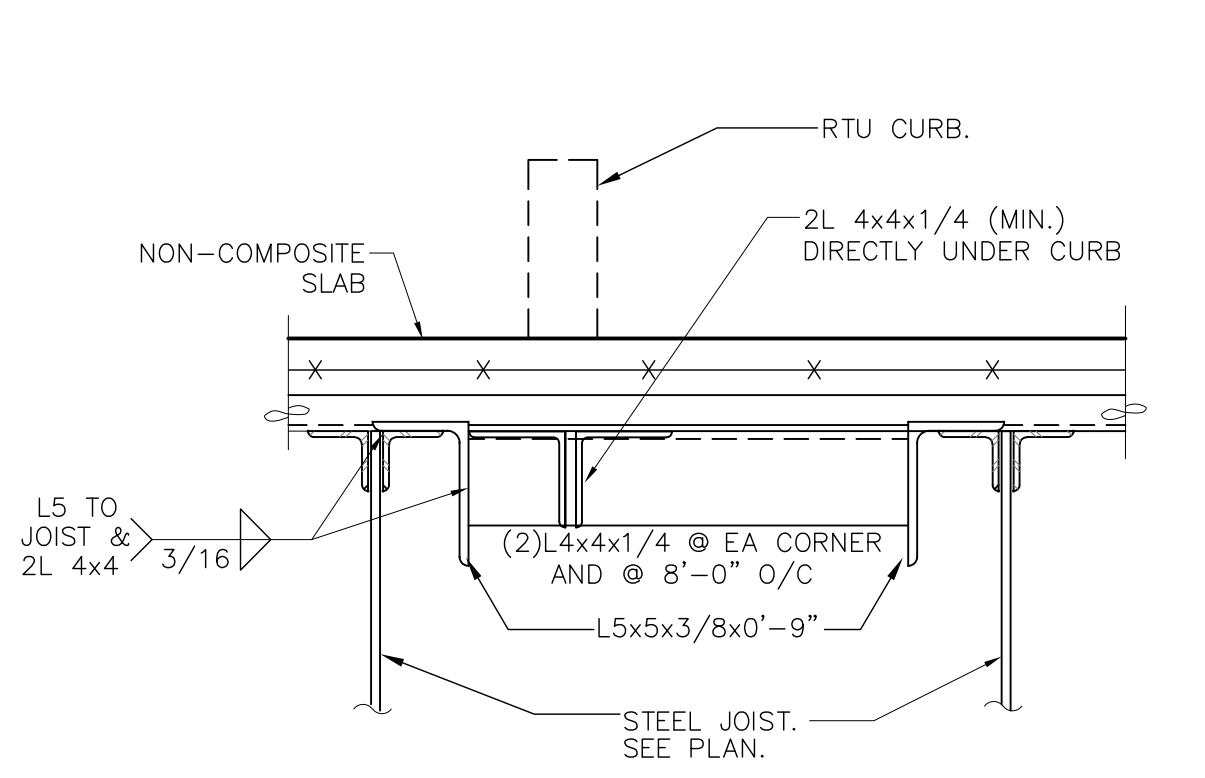
S1.1

SHEET NO.

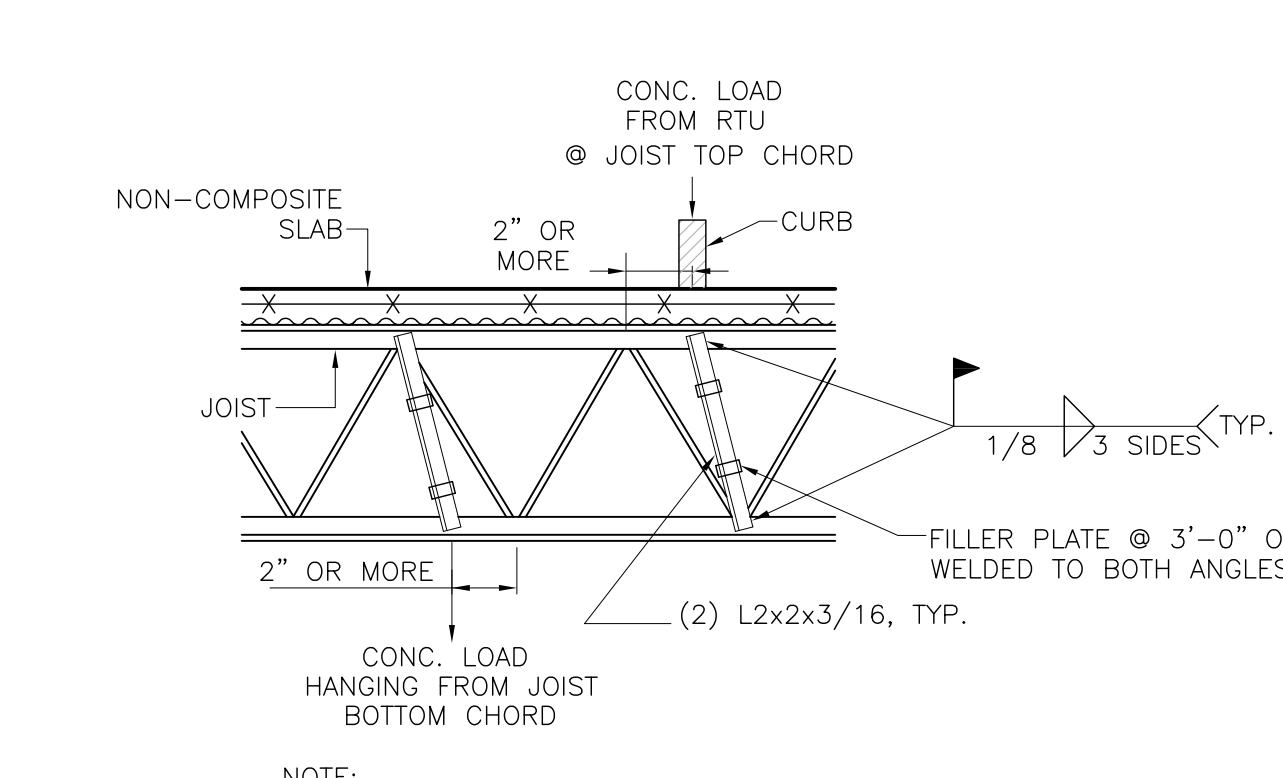
RTU SUPPORT DETAILS



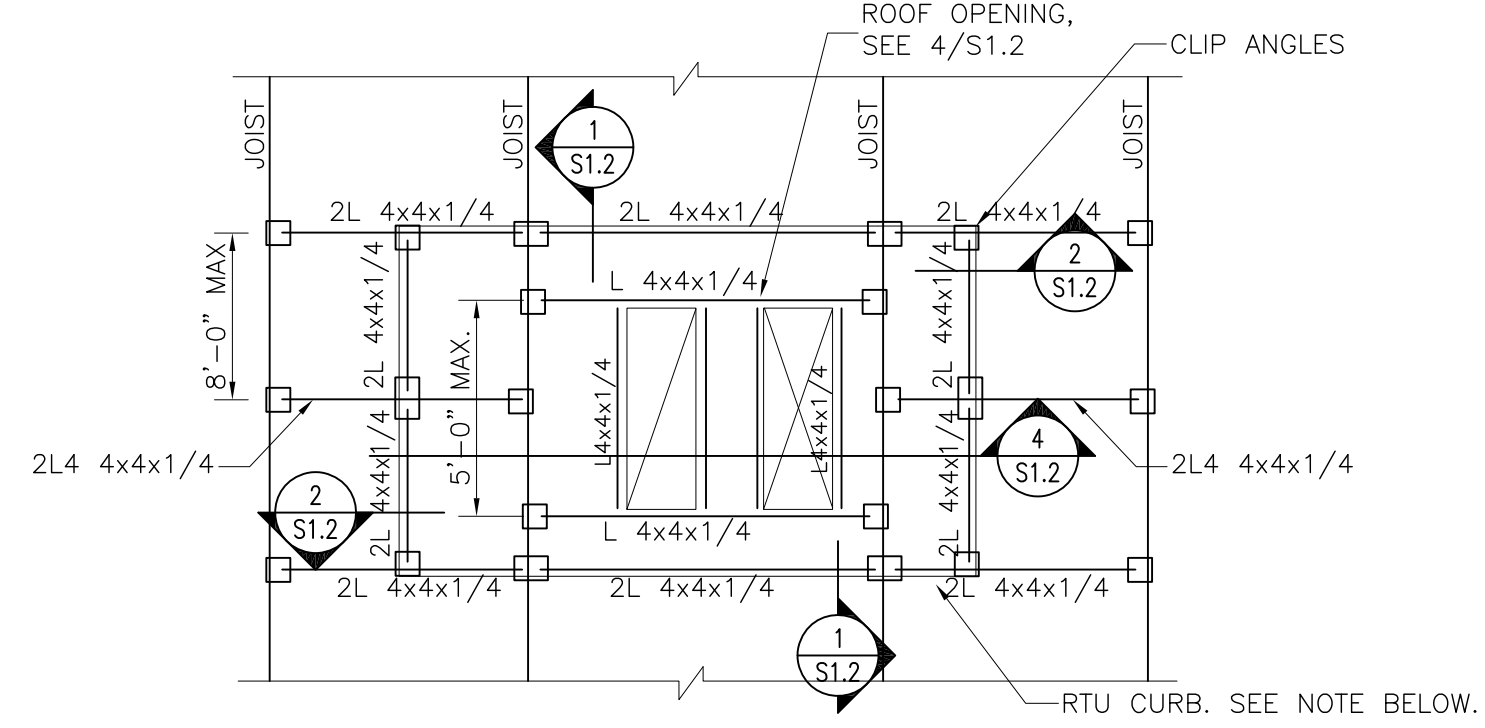
1 TYPICAL @ CURB-JOIST INTERSECTION



2 TYPICAL @ CURB PARALLEL TO JOIST



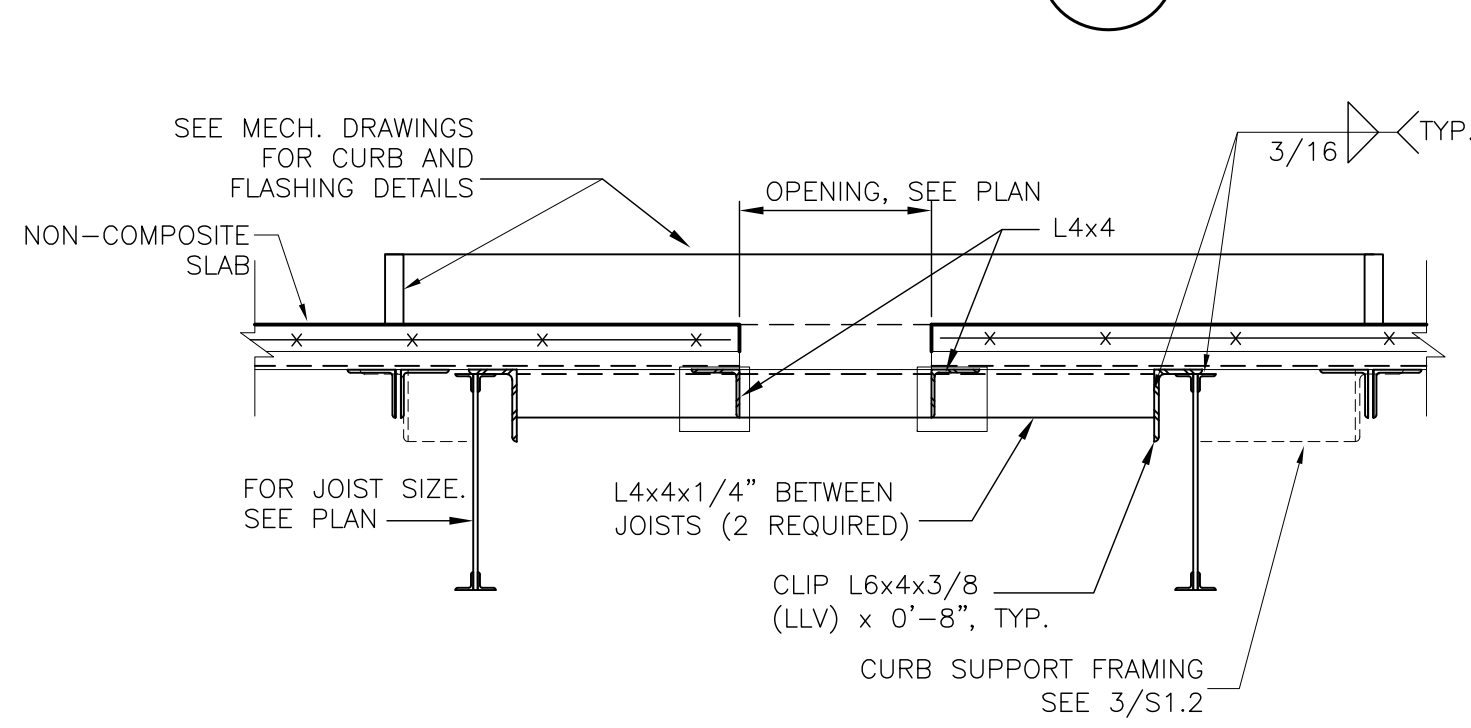
5 TYP. JOIST REINFORCEMENT @ LOADS AWAY FROM PANEL POINT
SCALE 3/4" = 1'-0"



PARTIAL PLAN @ RTU

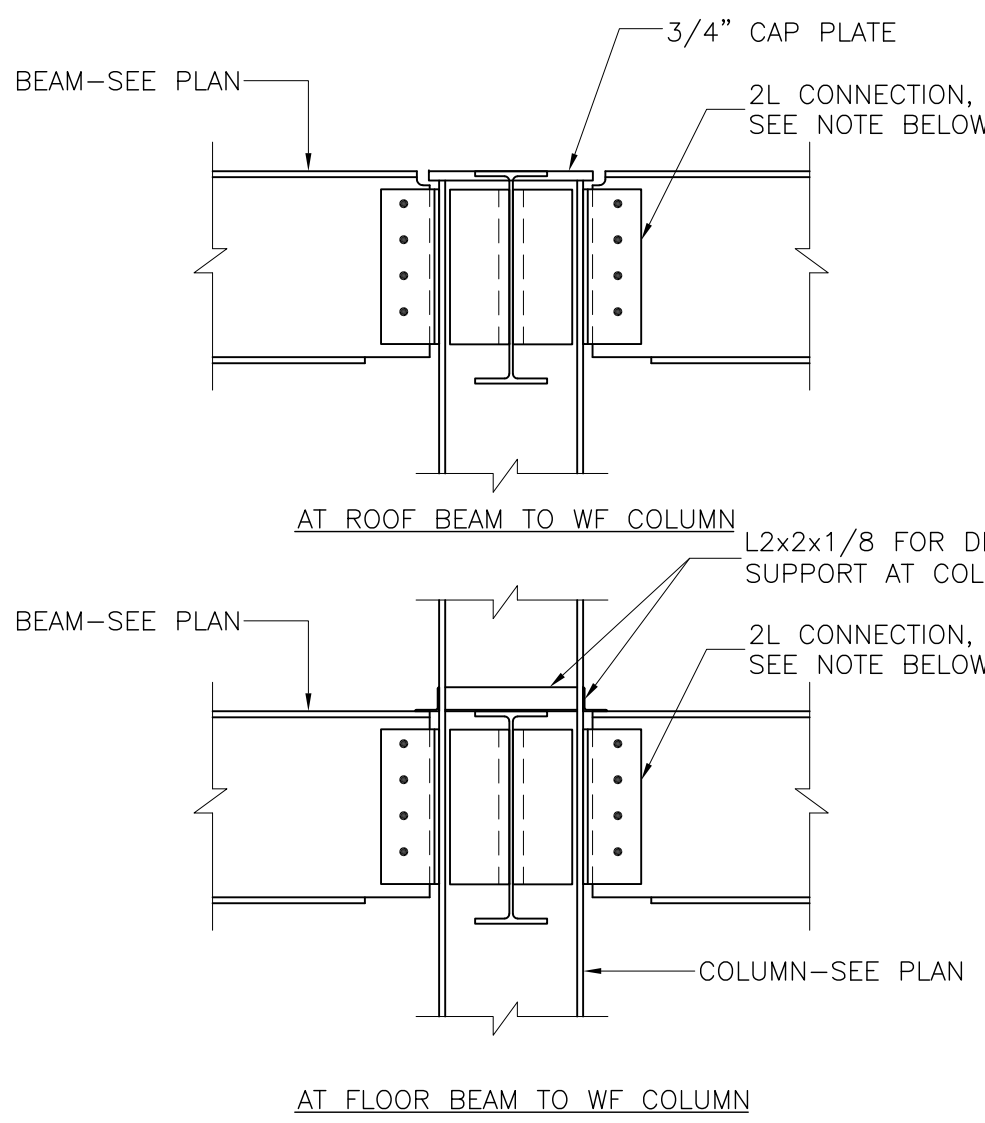
- NOTES:
- THE SLAB SHOULD NOT BE RELIED UPON TO SUPPORT THE UNIT WEIGHT. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO PROVIDE SECONDARY FRAMING REQUIRED FOR CURB SUPPORT. PROVIDE SHIMS AS REQUIRED TO PROVIDE LEVEL CURB.
 - PROVIDE TAPERED CURBS AS REQUIRED, WHERE FLOOR SLOPES.
 - WHERE THE FOOTPRINT OF THE RTU IS SMALLER THAN THE JOIST SPACING, SUCH THAT THE CURB IS BEARING ON SLAB ONLY, PROVIDE SUPPORT FRAMING UNDER THE ENTIRE CURB PERIMETER. SEE 2/S1.2.

3 RTU SUPPORT & OPENING FRAMING DETAIL



- NOTES:
- CONTRACTOR TO SUPPLY STEEL FABRICATOR WITH EXACT LOCATIONS AND DIMENSIONS OF EQUIPMENT AND ROOF OPENINGS.
 - PROVIDE SUPPORT ANGLES AT ALL SIDES OF OPENING.

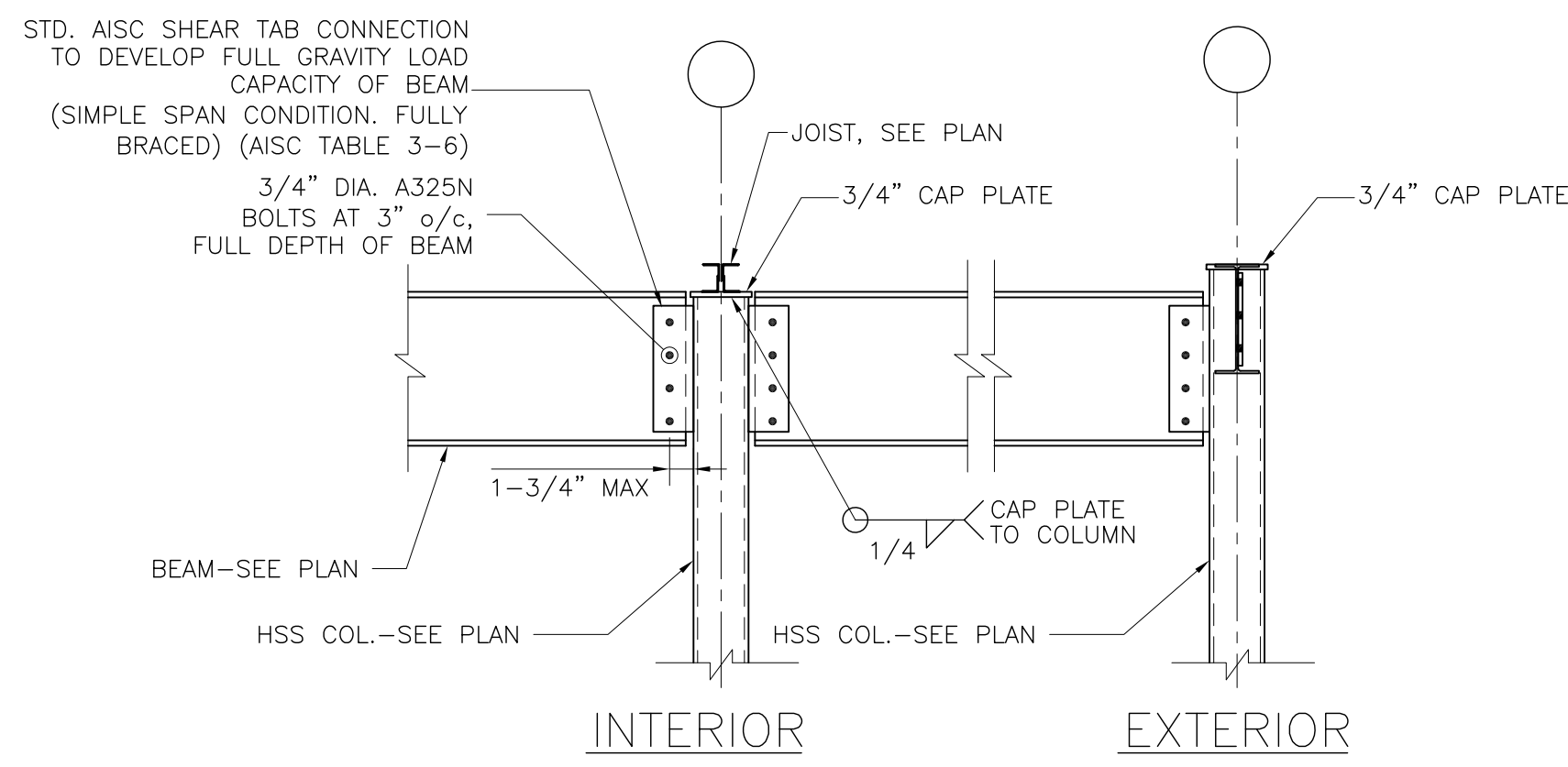
4 OPENING SECTION



A TYPICAL FRAMED CONNECTION, U.N.
(BEAM TO COLUMN)

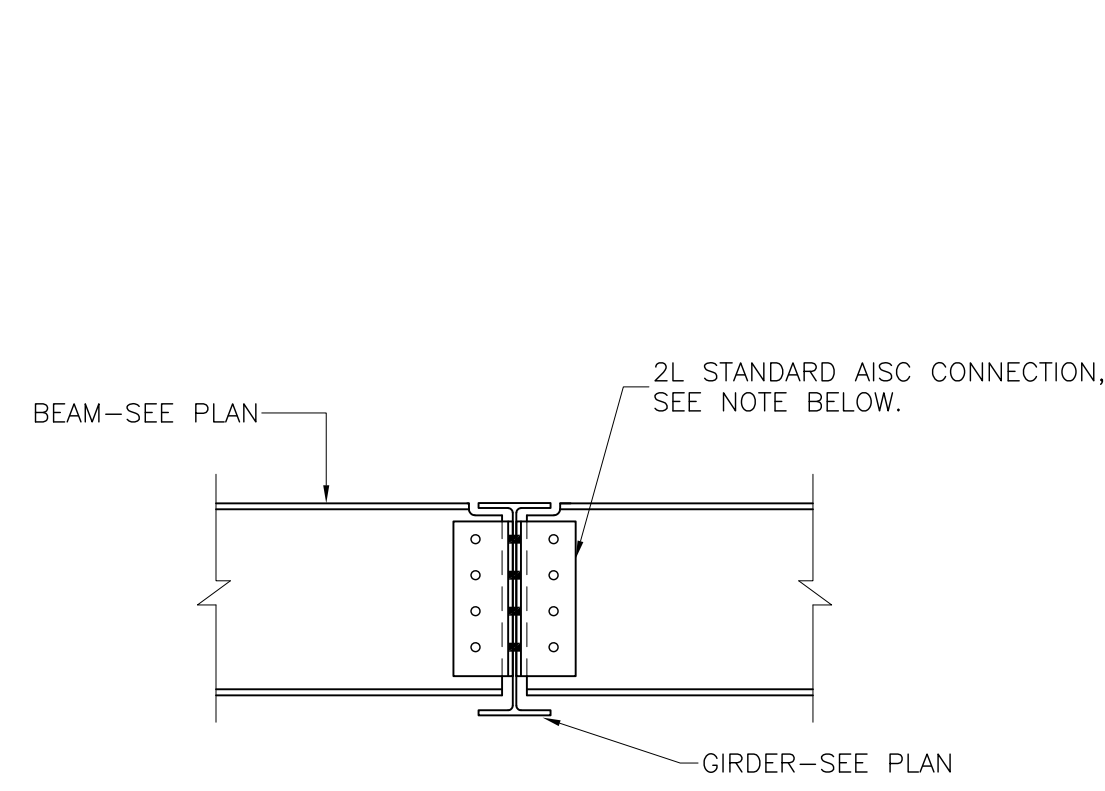
NOTE: FRAMED CONNECTION SHALL BE STANDARD AISC DOUBLE ANGLE CONNECTION DESIGNED TO SUPPORT THE GRAVITY LOAD CAPACITY OF THE CONNECTED BEAM. SEE GENERAL NOTES ON S1.0. THE BEAM SHALL BE ASSUMED FULLY BRACED.

10



B TYPICAL WIDE FLANGE BEAM-HSS COL. CONNECTION

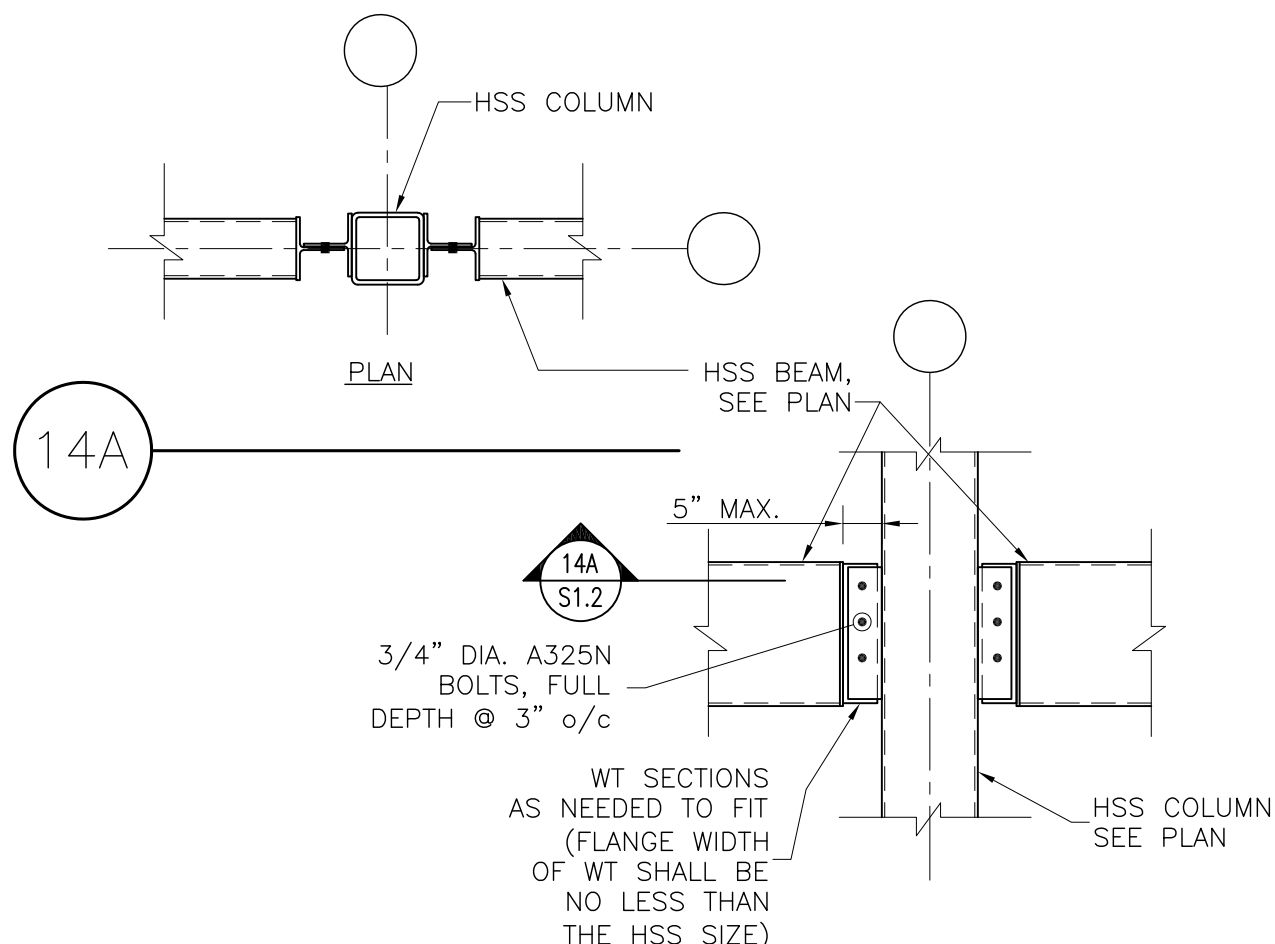
NOTES: SHEAR TAB CONNECTION SHOWN ABOVE SHALL BE USED FOR HSS COLUMN 7" WIDE OR LESS ONLY. PROVIDE 2 ANGLE FRAMED CONNECTION FOR HSS COLUMN 8" AND WIDER, DESIGNED BY STEEL DETAILER TO SUPPORT FULL GRAVITY LOAD CAPACITY OF THE BEAM. (AISC TABLE 3-6).



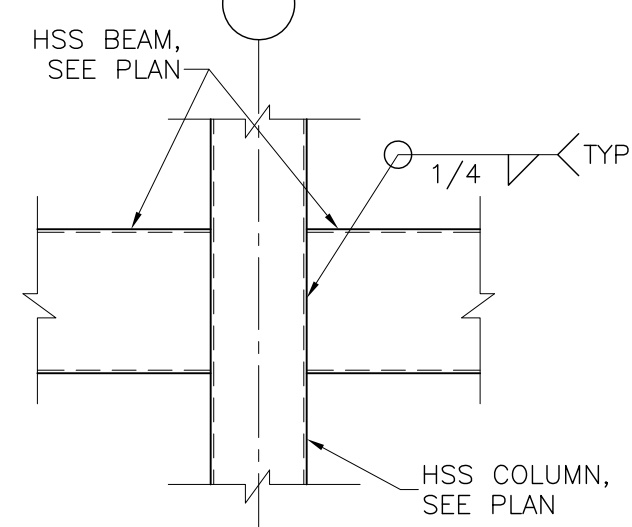
(SINGLE PLATE CONNECTION SHALL NOT BE PERMITTED, U.N. ON PLANS)

13 TYPICAL BEAM TO BEAM CONNECTION, U.N.

NOTE: FRAMED CONNECTION SHALL BE STANDARD AISC DOUBLE ANGLE CONNECTION DESIGNED TO SUPPORT THE GRAVITY LOAD CAPACITY OF THE CONNECTED BEAM. SEE GENERAL NOTES ON S1.0. THE BEAM SHALL BE ASSUMED FULLY BRACED.



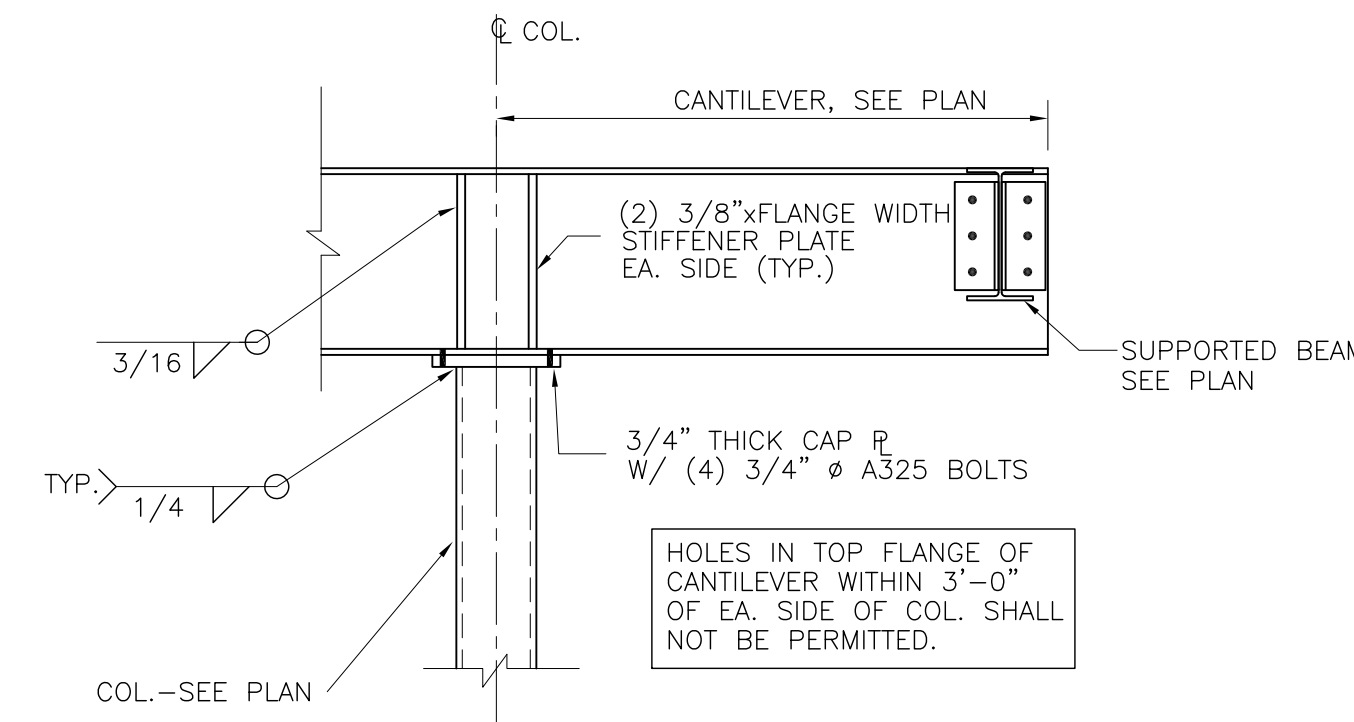
A. SUPPORTING GRAVITY LOADS ONLY



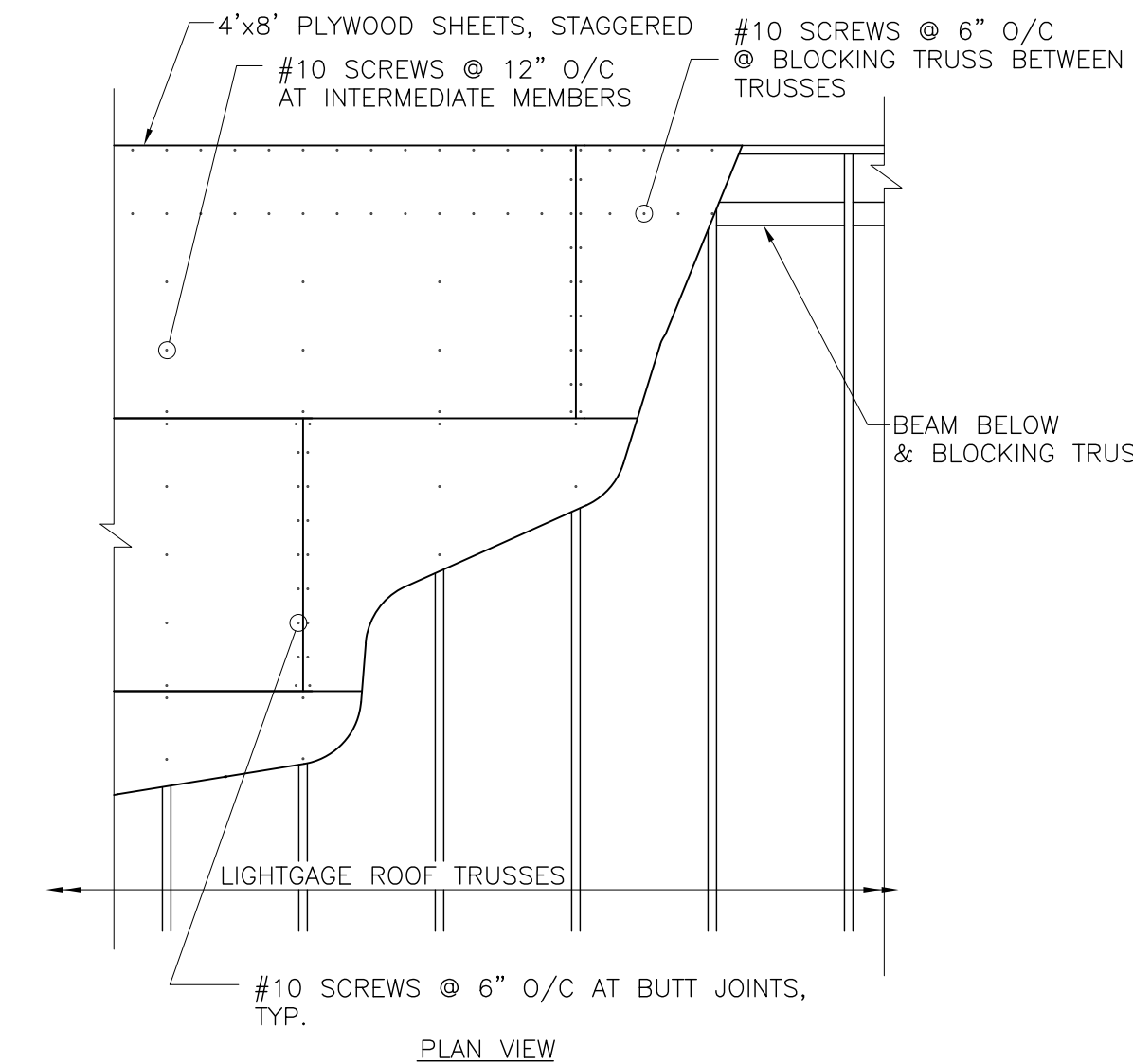
ALL STRUCTURAL STEEL EXPOSED TO VIEW SHALL BE DETAILED AND FABRICATED TO MEET AESS STANDARDS.

B. SUPPORTING WIND & GRAVITY LOADS

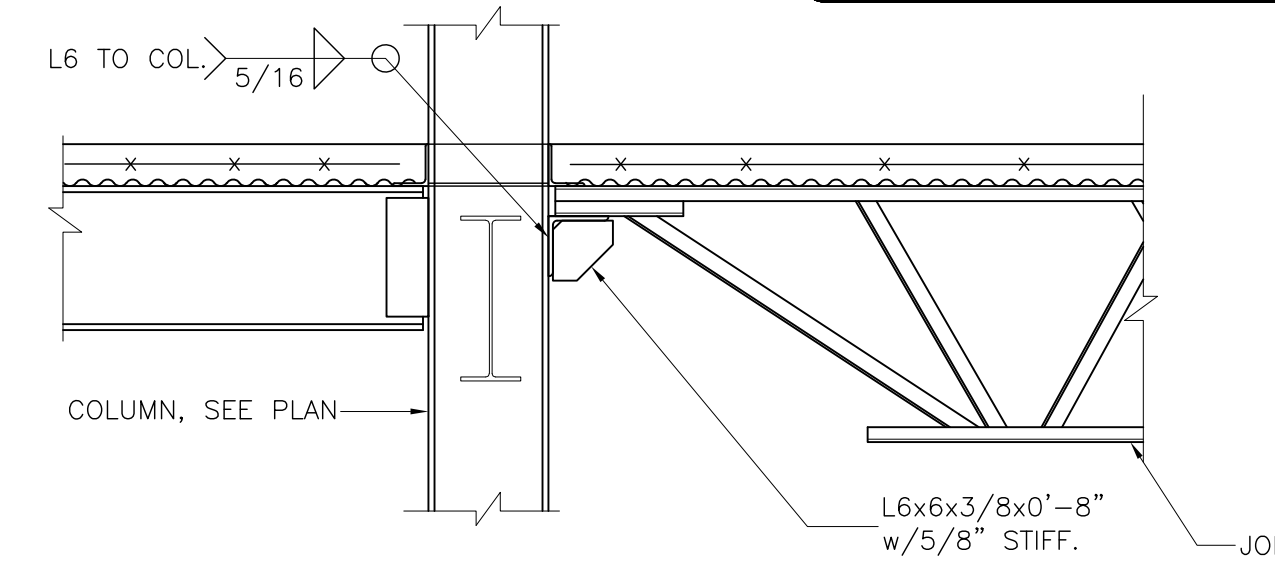
14 TYPICAL HSS BEAM TO HSS COLUMN CONNECTION



6 TYPICAL CANTILEVER, U.N.



8 PLYWOOD ROOF DECK ATTACHMENT DETAILS



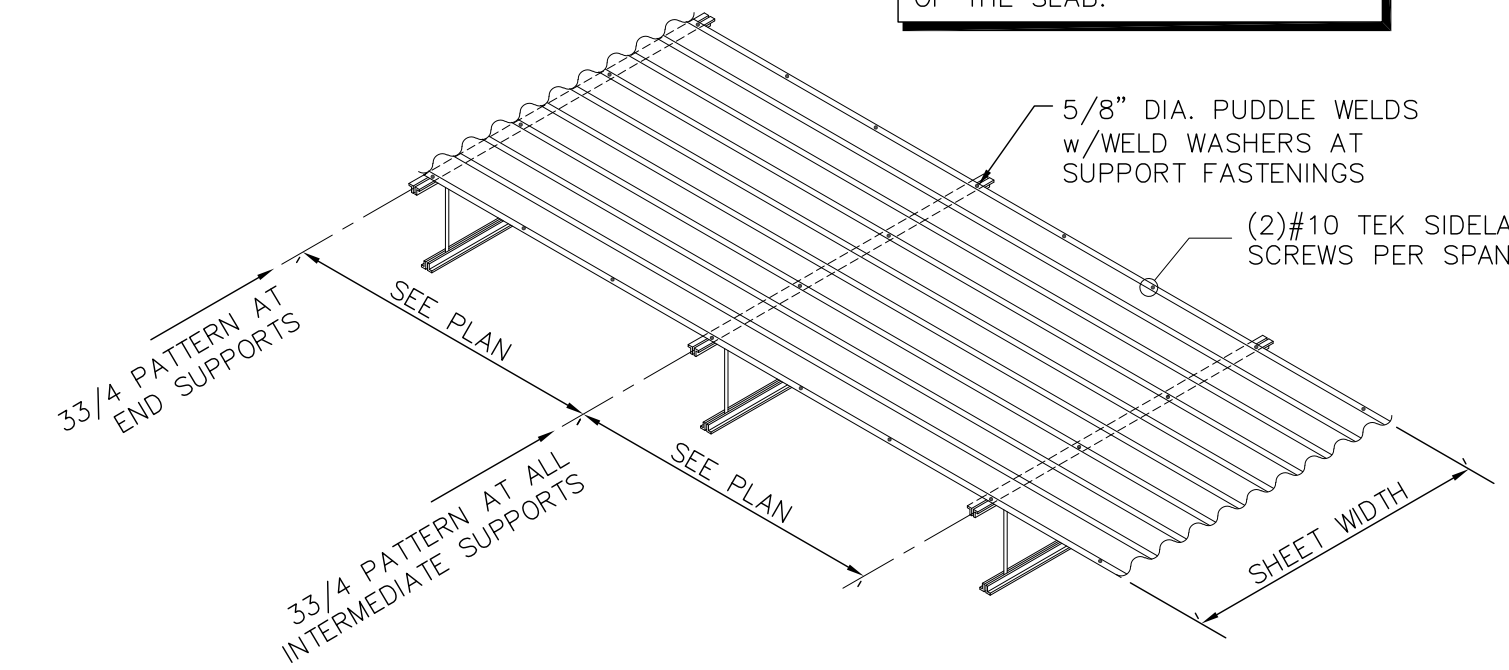
7 TYPICAL JOIST/COLUMN CONNECTION

NOTES:

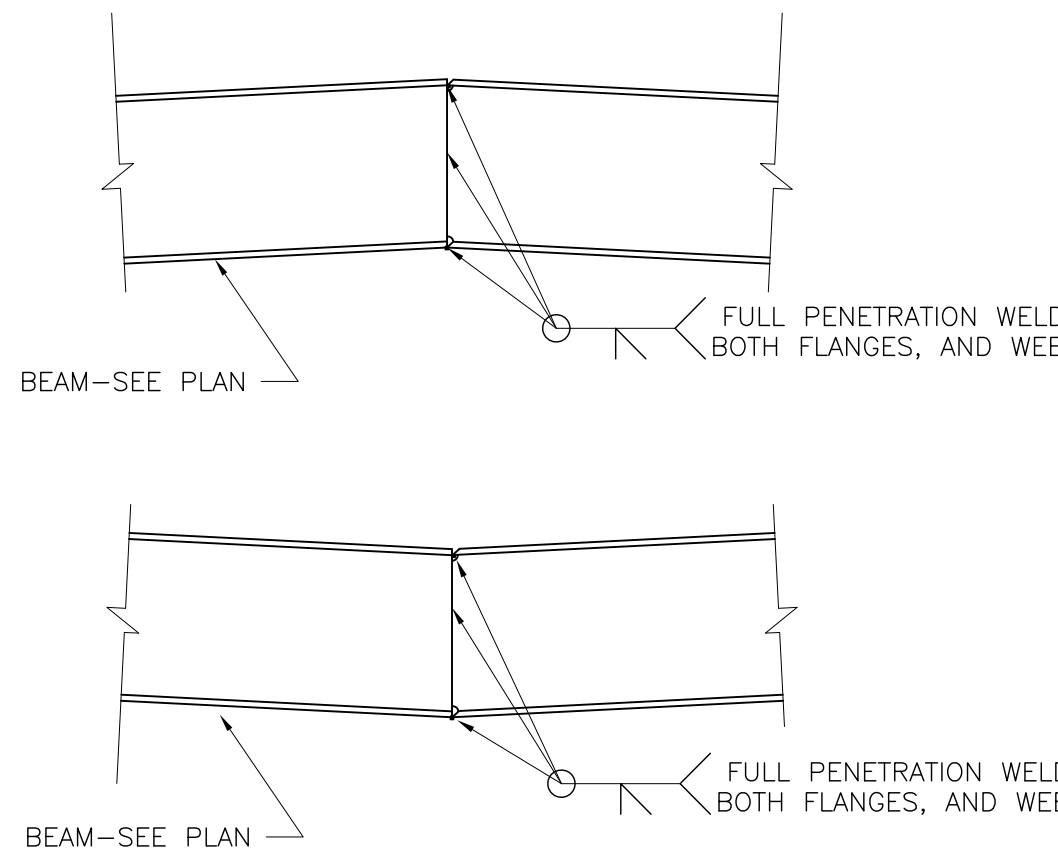
- ANCHOR THROUGH MULTIPLE SHEETS AT ALL END LAPS.
- ANCHOR REQUIRED AT ALL SIDE LAP SUPPORT POINTS. END LAPS SHALL OCCUR ONLY AT SUPPORT POINTS.
- PROVIDE 5/8" DIA. PUDDLE WELDS w/WASHERS @ 6" O.C. AT PERIMETER OF FLOOR.

FORMDECK SHALL BE EQUAL TO OR BETTER THAN VULCRAFT 1.00 CONFORM DECK (GALVANIZED). SEE PLAN NOTES FOR THICKNESS. MAINTAIN AT LEAST THREE SPAN CONDITION.

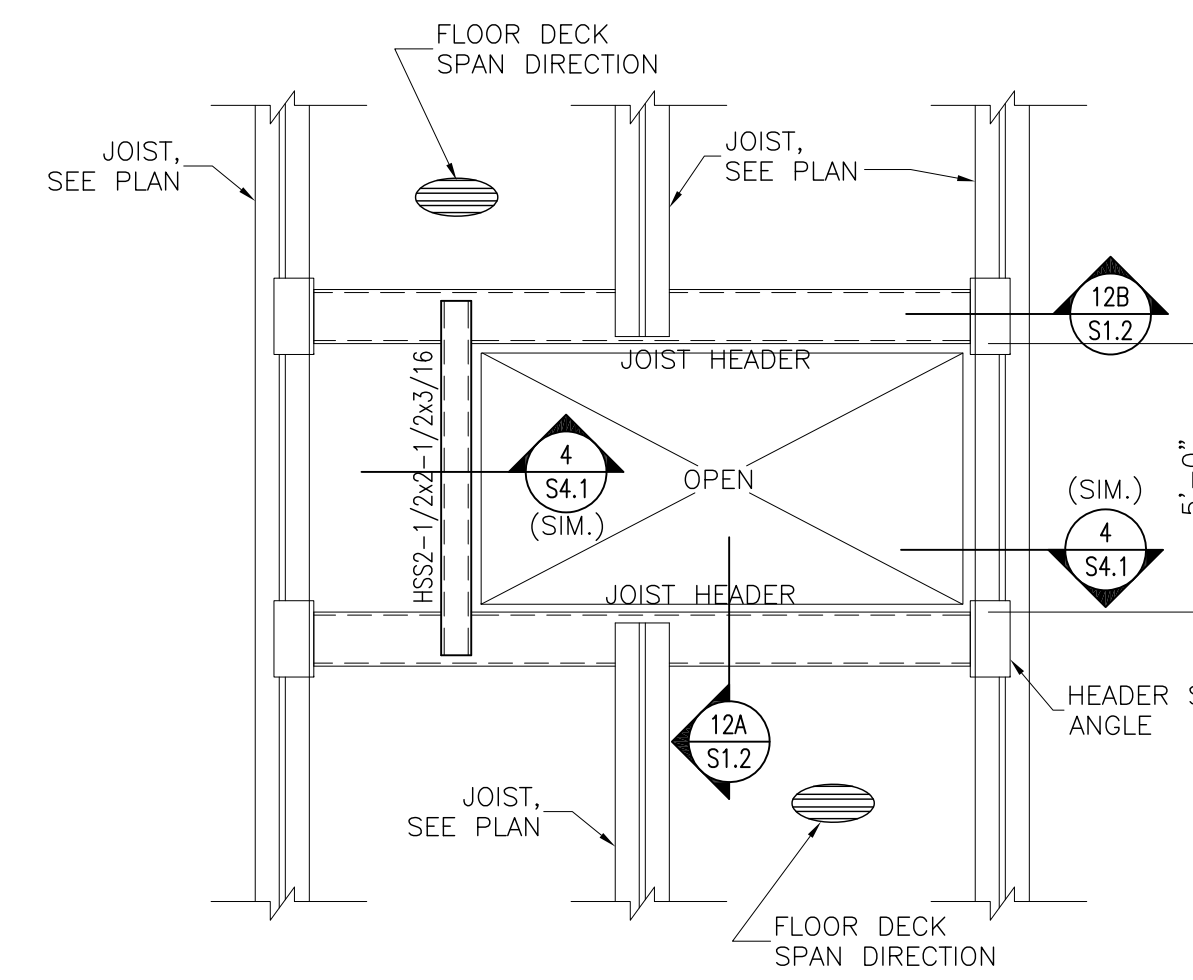
WELDED WIRE FABRIC IS TO BE PLACED ON CHAIRS SO THAT IT IS LOCATED 1" FROM THE TOP OF THE SLAB.



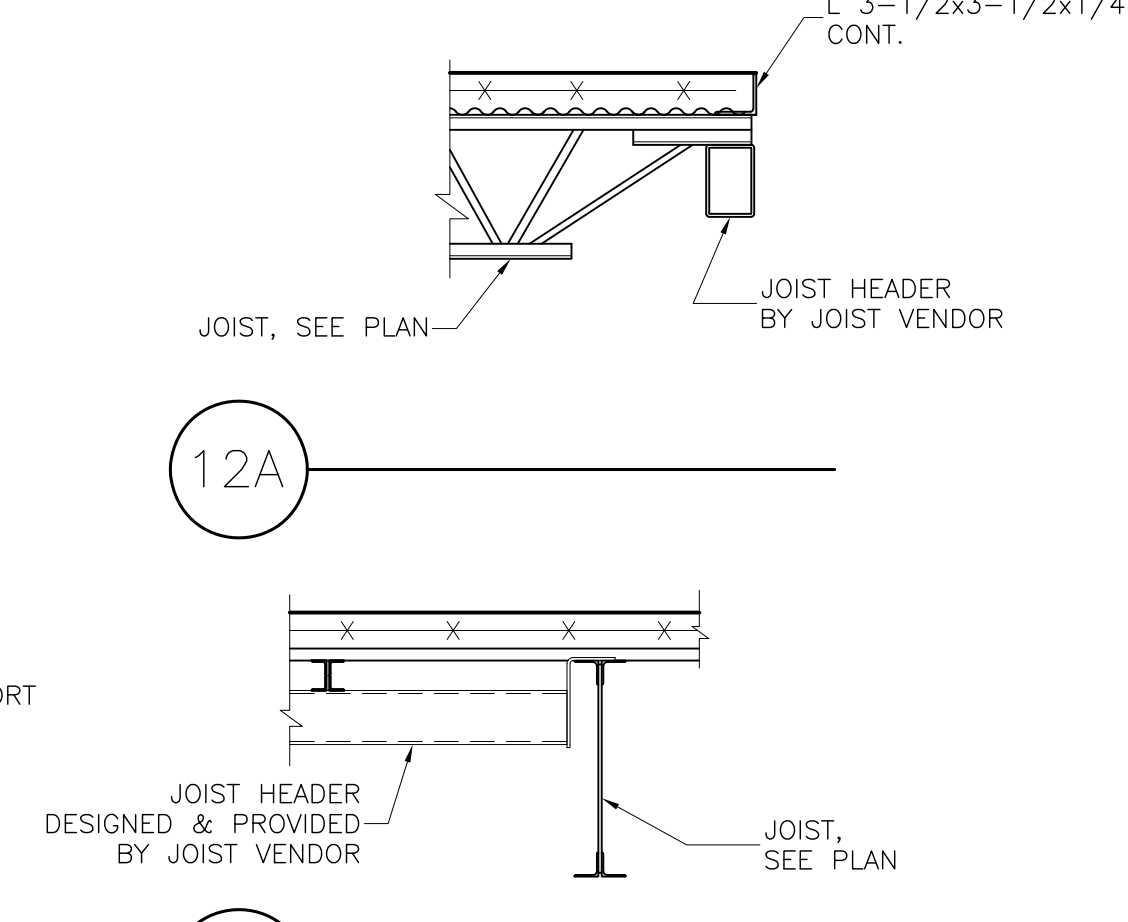
9 TYPICAL FLOOR DECK CONNECTION DETAIL
NO SCALE



11 CRANKED BEAM DETAIL
SCALE 3/4" = 1'-0"



12 TYPICAL @ FLOOR CHASE OPENING
NTS



12A

12B

TOM ROBISON & ASSOCIATES, INC.
STRUCTURAL ENGINEERS
1716 KIRBY PARKWAY - SUITE 201
MEMPHIS, TENNESSEE 38120
901/754-4411
sarfraz_k@bellsouth.net

PROJECT NO. 160103
DATE: 05-21-16
DRAWN BY: RC

REVISION:
DATE:

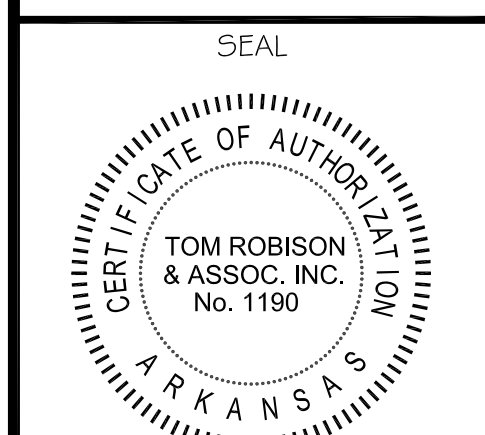
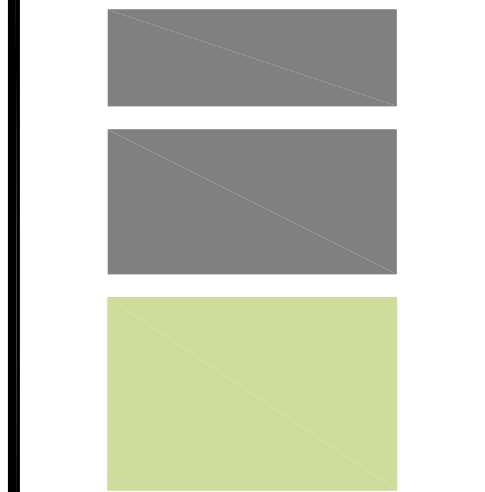
NOTICE:
THIS DRAWING IS THE PROPERTY OF MATT SILAS ARCHITECT ONLY TO BE USED SOLELY FOR THIS PROJECT.
COPYRIGHT © 2018

MSA
MATT SILAS ARCHITECT
Jonesboro, Arkansas 72401
Tel: (870) 268-0501
Fax: (870) 268-0500
Email: mattsilas@beglobal.net
Website: mattsilasarchitect.com

CENTRAL BAPTIST
CHURCH
ADMINISTRATION
WING ADDITION

3707 HARRISBURG ROAD
JONESBORO, ARKANSAS
72404

centralbaptist

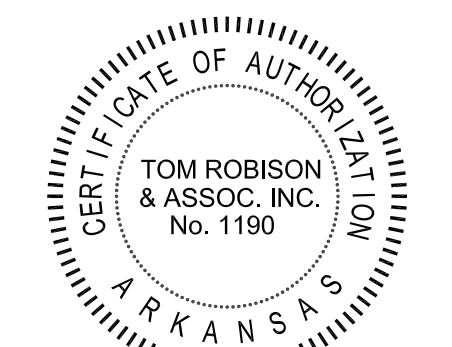


TYPICAL DETAILS

S1.2
SHEET NO.



SEAL



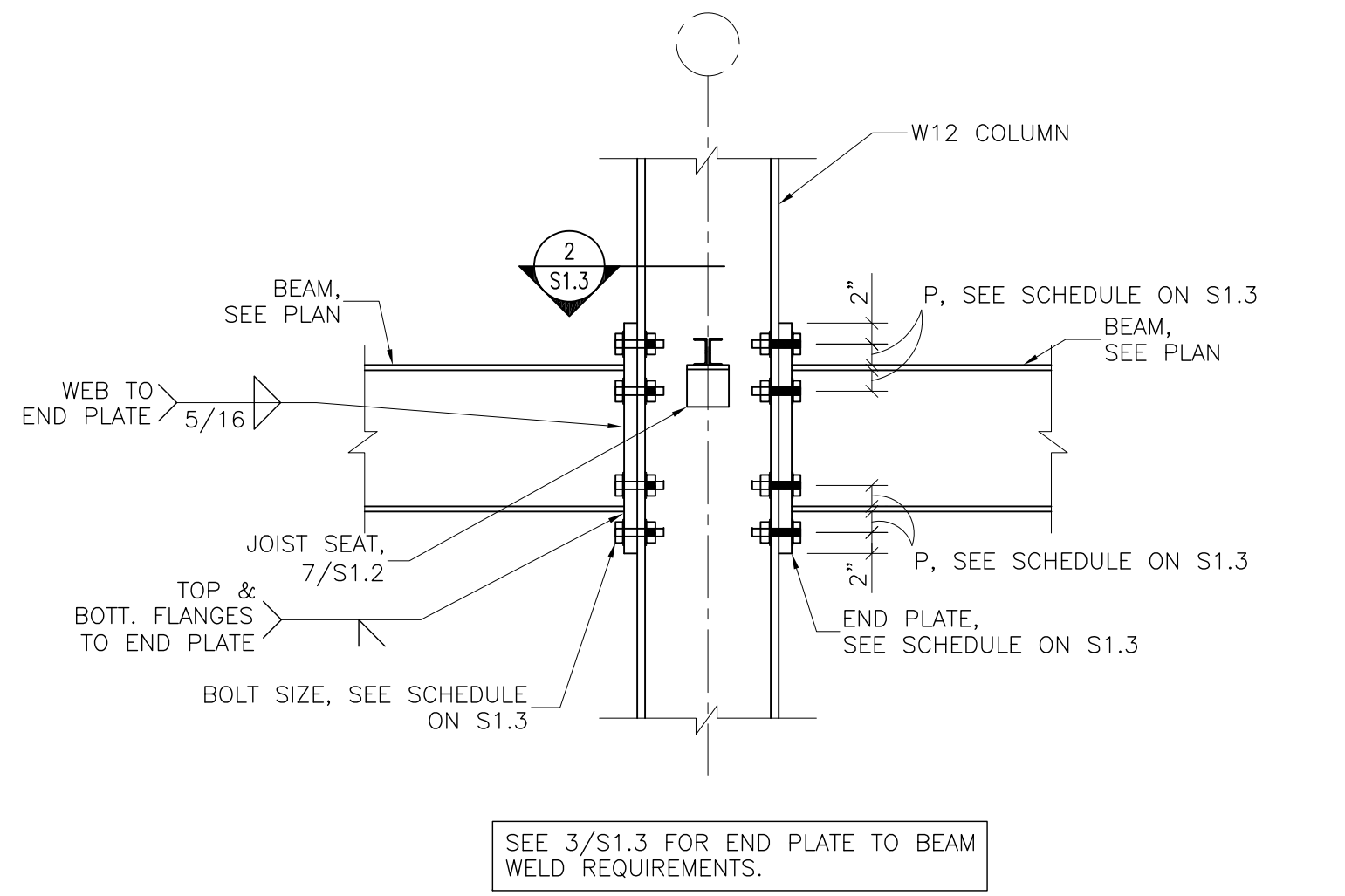
SEAL



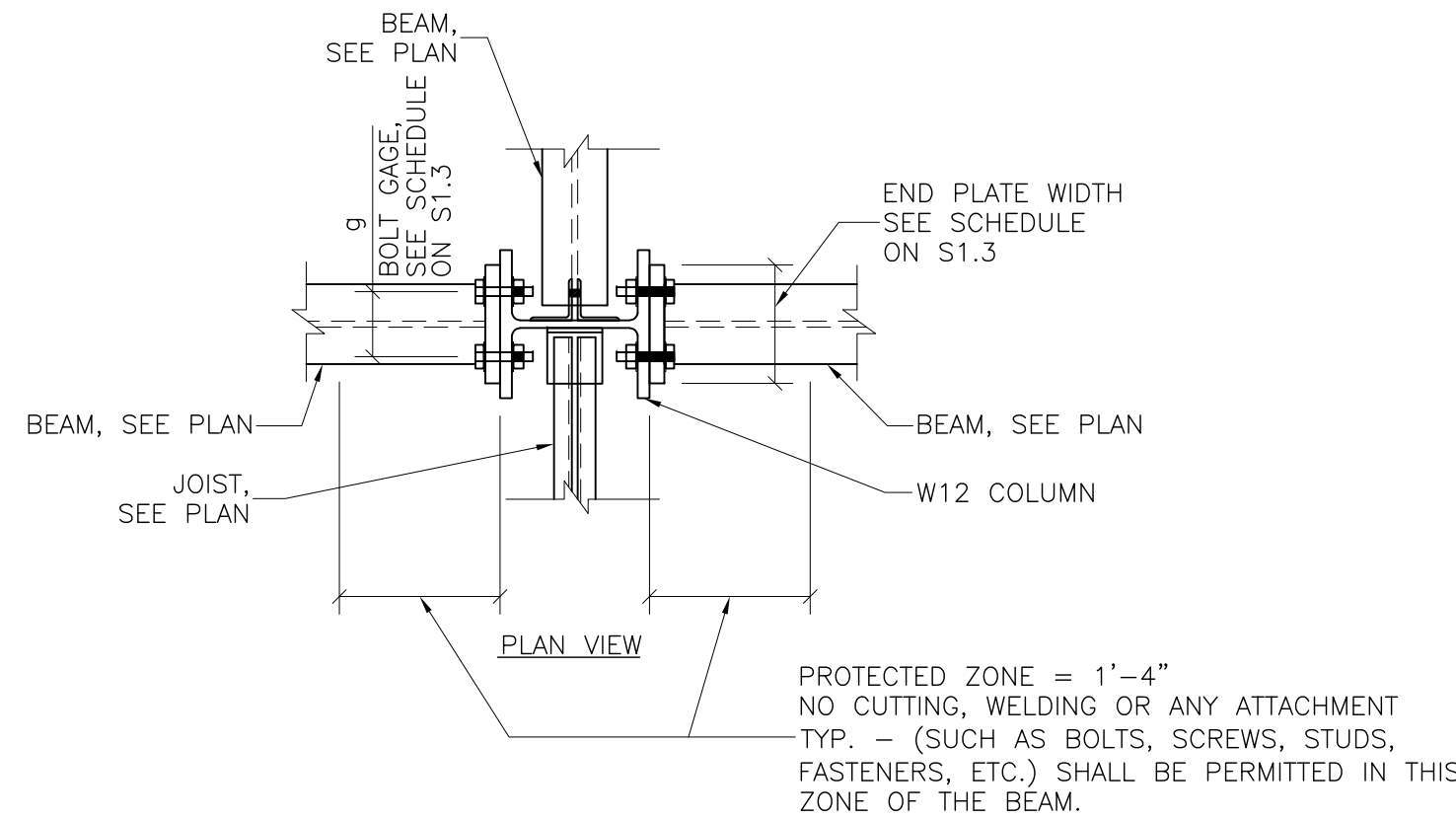
TYPICAL DETAILS

S1.3

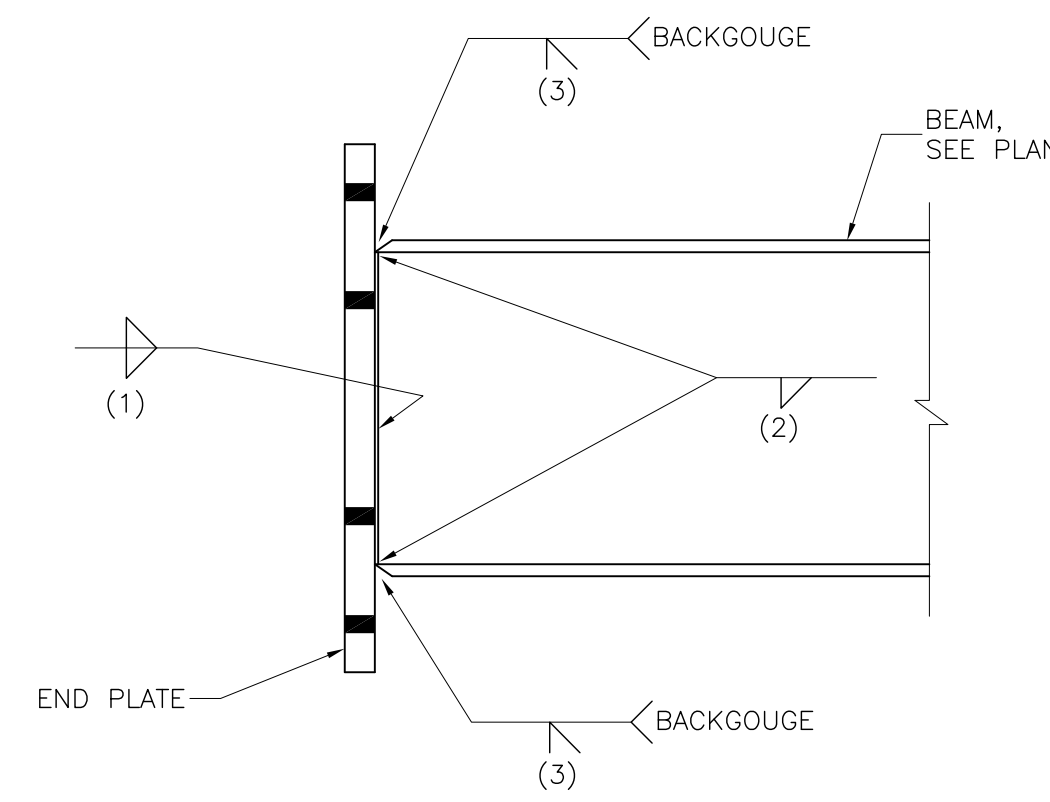
SHEET NO.



1 MOMENT CONNECTION DETAIL @ FLOOR
SCALE 3/4" = 1'-0"



2 MOMENT CONNECTION, TYP.
SCALE 3/4" = 1'-0"



3 END PLATE TO BEAM WELDING REQUIREMENTS

NOTES:

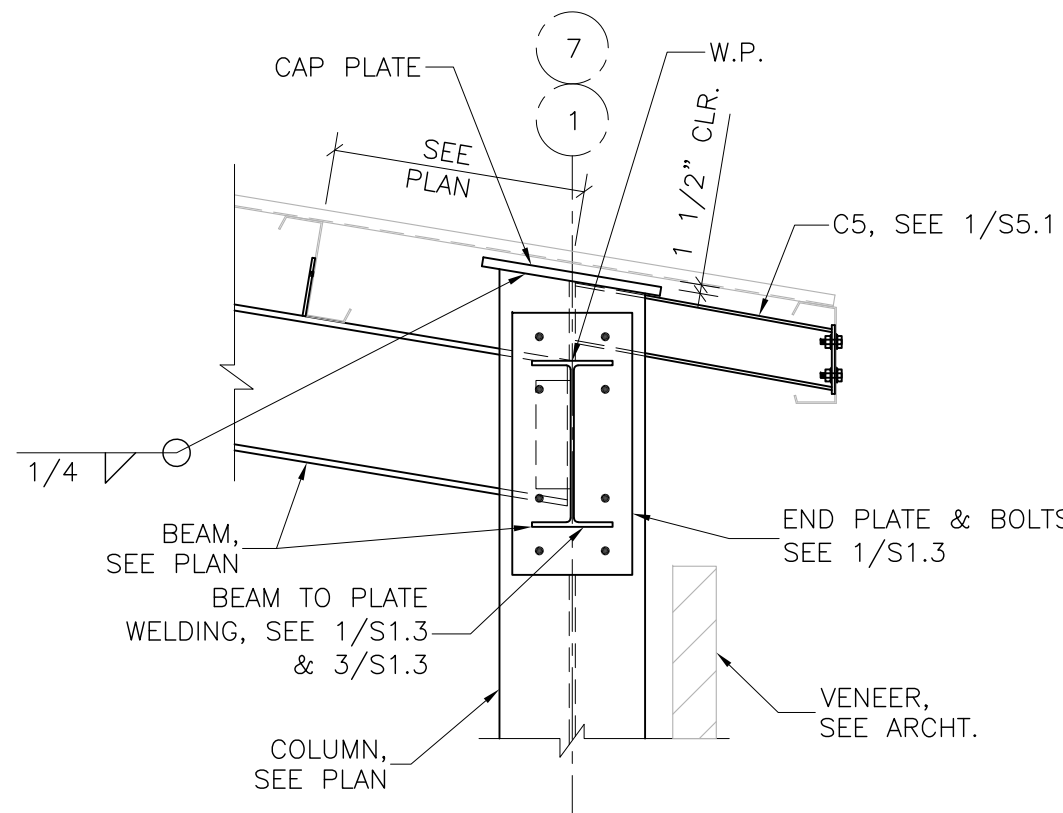
- PREPARE THE FLANGES OF THE BEAM WITH A 45 DEGREE, FULL DEPTH BEVEL.
- FIT UP THE END PLATE AND BEAM WITH A MINIMUM ROOT OPENING.
- PREHEAT THE SPECIMENS AS REQUIRED BY AWS SPECS.
- PREPARE THE SURFACES FOR WELDING AS REQUIRED BY AWS SPECS.
- PLACE THE WEB WELDS (1).
- PLACE THE 5/16" BACKING FILLET WELDS ON THE BEAM WEB SIDE OF THE BEAM FLANGES (2). WELDS PLACED IN ERROR SHALL BE REPAIRED AS REQUIRED BY AISC-358.
- BACKGOUGE THE ROOT OF THE BEVEL TO REMOVE ANY CONTAMINENTS FROM THE 5/16" BACKER FILLET WELDS (3).
- PLACE THE FLANGE GROOVE WELDS (AWS TC-U4b-GF).
- BACKER BARS OR ACCESS HOLES SHALL NOT BE USED.
- REMOVE WELD TABS.
- PLATE TO FLANGE WELDS SHALL BE DEMAND CRITICAL. SEE AWS D1.8 FOR REQUIREMENTS.
- PROVIDE INSPECTIONS AND TESTING AS REQUIRED BY AISC 341.

EXTENDED END PLATE SCHEDULE

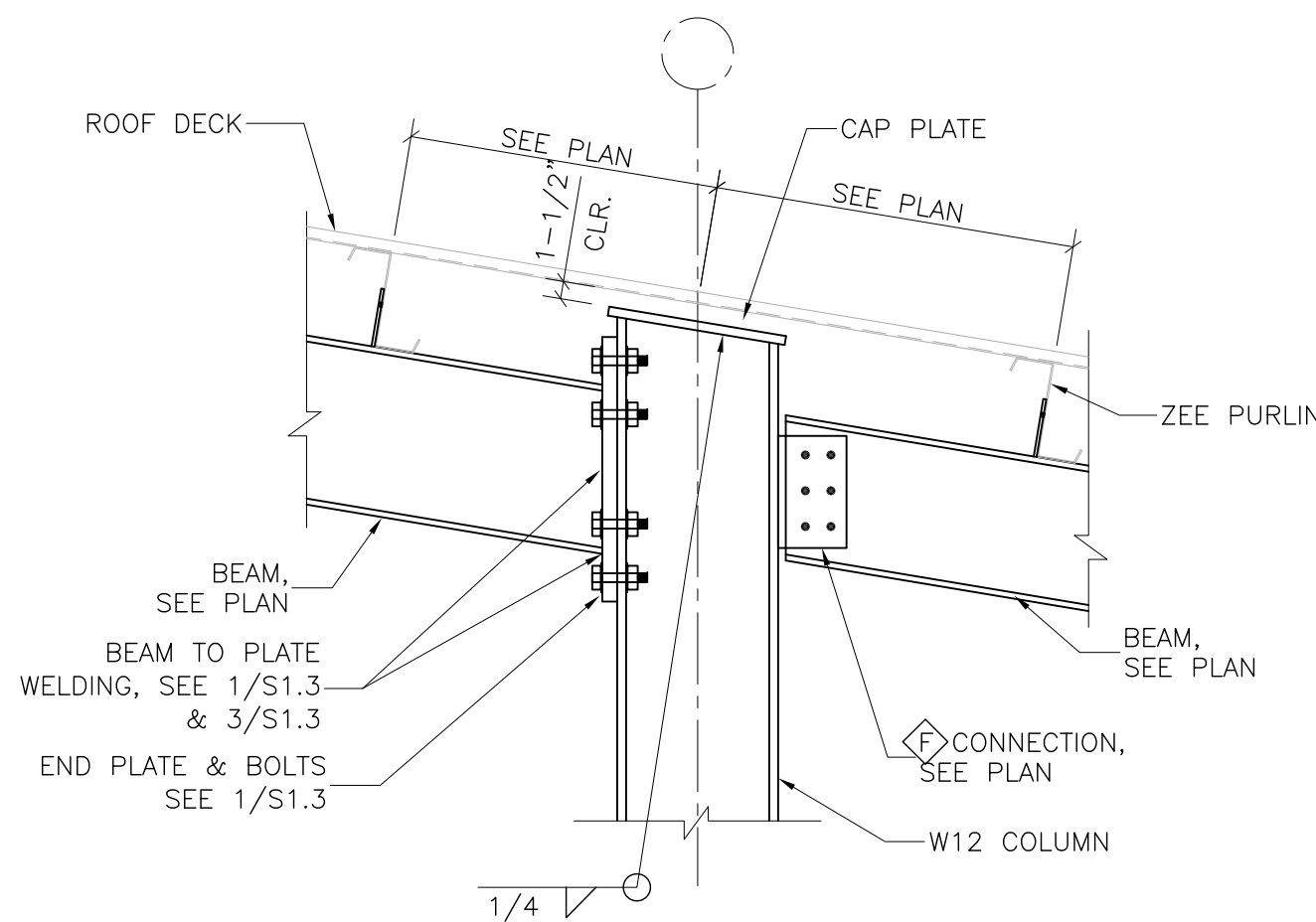
BEAM SIZE	BOLT Ø	END PLATE (WIDTH x THICKNESS)	BOLT GAGE (g)	BOLT PITCH (p)
W14x30	1" DIA	7 1/2"x1"	5 1/2"	1 1/2"
W14x34	1" DIA	7 1/2"x1"	5 1/2"	1 1/2"
W14x38	1 1/8" DIA	8"x1 1/8"	5 1/2"	1 7/8"
W16x36	1" DIA	8"x1"	5 1/2"	1 1/2"
W16x40	1 1/8" DIA	8"x1 1/8"	5 1/2"	1 7/8"
W16x45	1 1/8" DIA	8"x1 1/8"	5 1/2"	1 7/8"
W16x50	1 1/4" DIA	8"x1 1/4"	5 1/2"	2"

NOTES:

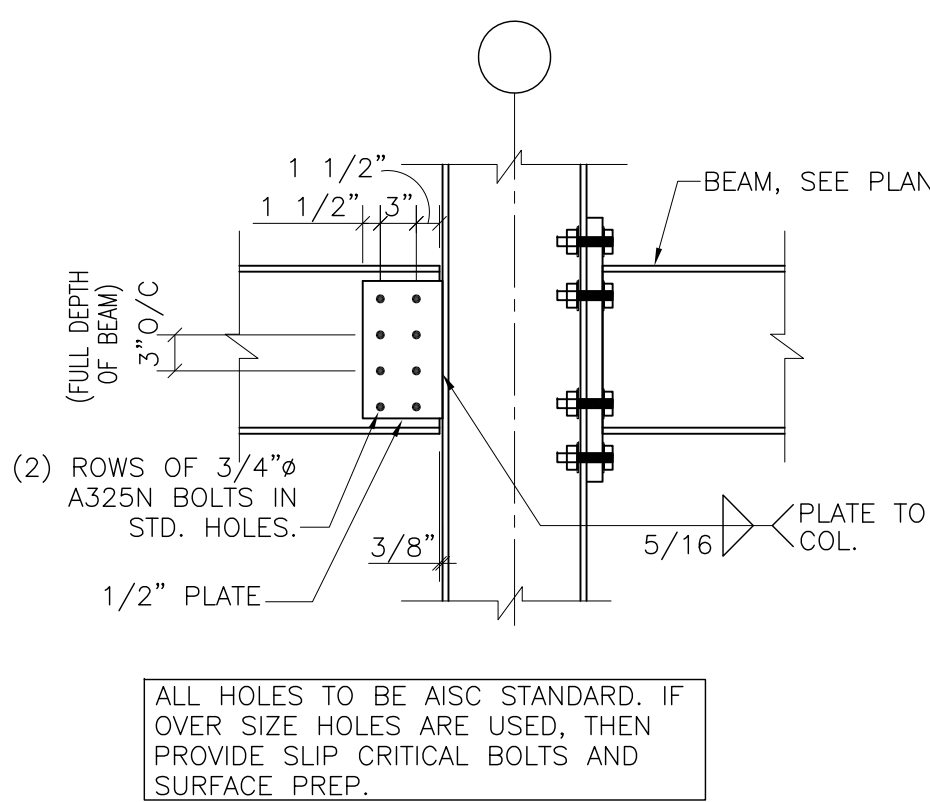
1. ALL BOLTS @ MOMENT CONN. TO BE A490X PRE-TENSIONED. PROVIDE DIRECT TENSION INDICATOR UNDER EACH UNIT. FULL FINGER SHIMS MAY BE USED IF NEEDED. FAYING SURFACES SHALL BE PREPARED AS CLASS A SLIP CRITICAL.
2. END PLATE MATERIAL = ASTM A572 (GR. 50 KSI)
3. SEE 3/S1.3 FOR BEAM TO END PLATE WELDING
4. MAX. BOLT HOLES = db + 1/16"



4 MOMENT CONNECTION @ ROOF (EXTERIOR) COLUMN



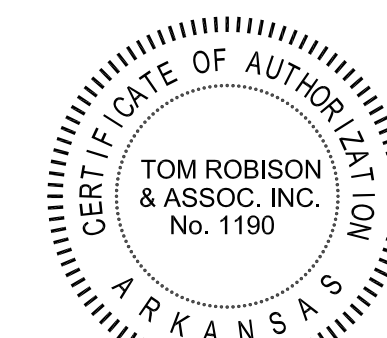
5 MOMENT CONNECTION @ ROOF (INTERIOR) COLUMN



6 DRAG BEAM TO BEAM CONNECTION TYPE F
NTS



SEAL



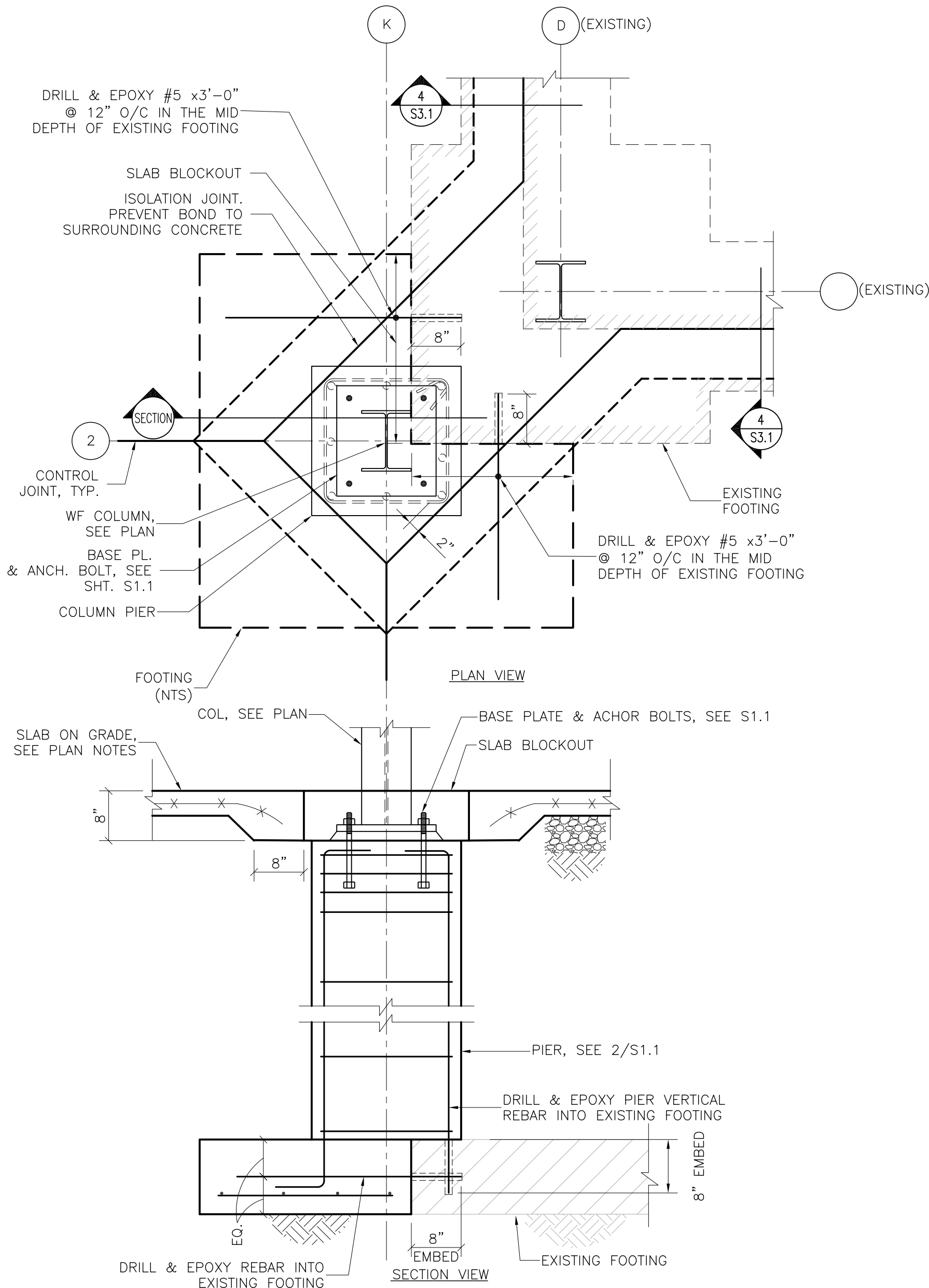
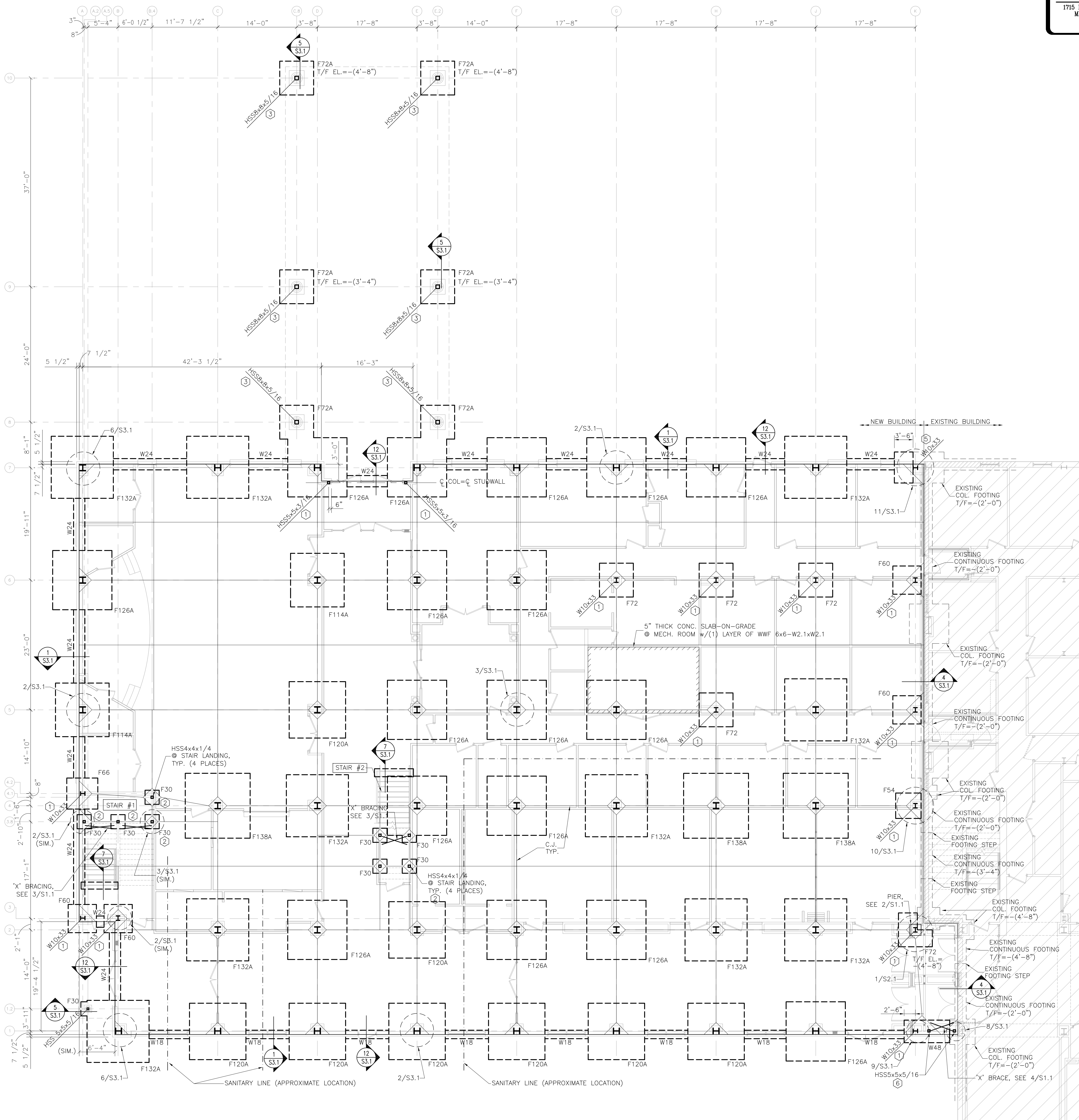
SEAL



FOUNDATION AND
GROUND FLOOR PLAN

S2.1

SHEET NO.



PLAN NOTES:

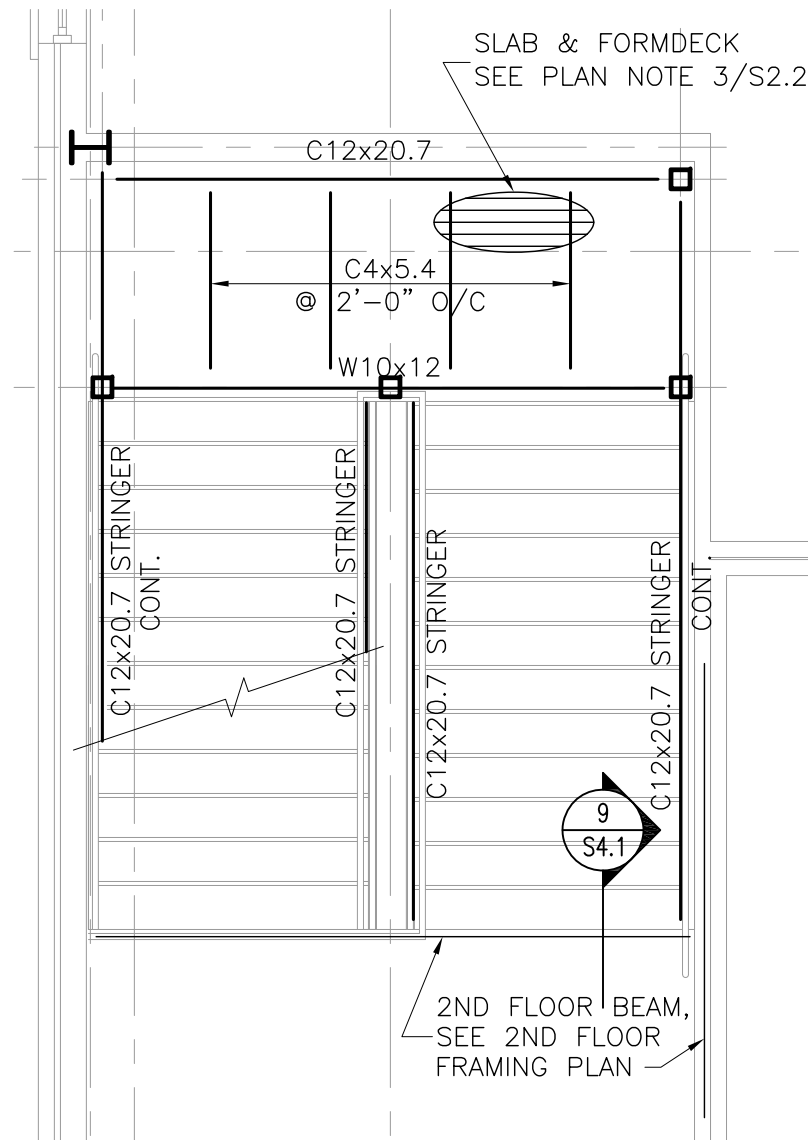
- SEE SHEET S1.0 THRU S1.2 FOR GENERAL NOTES, SCHEDULES AND TYPICAL DETAILS.
- FINISH FLOOR SLAB ELEV. TO MATCH EXISTING FLOOR SLAB = 0'-0" DATUM.
- ALL ELEVATIONS ARE REFERENCED TO THIS DATUM.
- 4" SLAB ON GRADE W/(1) LAYER WWF 6x6-W1.4xW1.4 (Ø 1" FROM TOP OF SLAB) TYP. UNLESS NOTED (SEE 1/S1.0)
- ALL FOOTINGS ARE CENTERED ON COLUMNS OR WALLS UNLESS NOTED

- TOP OF COLUMN AND WALL FOOTING ELEVATION:
A. INTERIOR: -(1'-0") TYP. U.N.
B. EXTERIOR: -(2'-0") BELOW FIN. FLOOR, U.N. (SEE PLAN)
C. AT EXISTING BUILDING, MATCH BOTTOM OF NEW FOOTING TO BOTTOM OF EXISTING FOOTING. DO NOT UNDERMINE EXISTING FOOTING. FIELD VERIFY TOP OF EXISTING FOOTING SHOWN.
- DEPRESS FOOTINGS AS REQUIRED IF PLUMBING OR OTHER LINES MUST PASS OVER FOOTINGS. SEE PIPING DETAILS ON S1.1 IF FOOTING IS DEPRESSED PROVIDE COLUMN PEDESTAL TO ELEVATION -(0'-8"). SEE PIER 2/S1.1.
- FOR FLOOR DRAINS AND SLOPES SEE ARCHT. DWGS.
- ALL COLUMNS SHALL BE W12x136, EXTENDED TO ROOF, UNLESS NOTED.

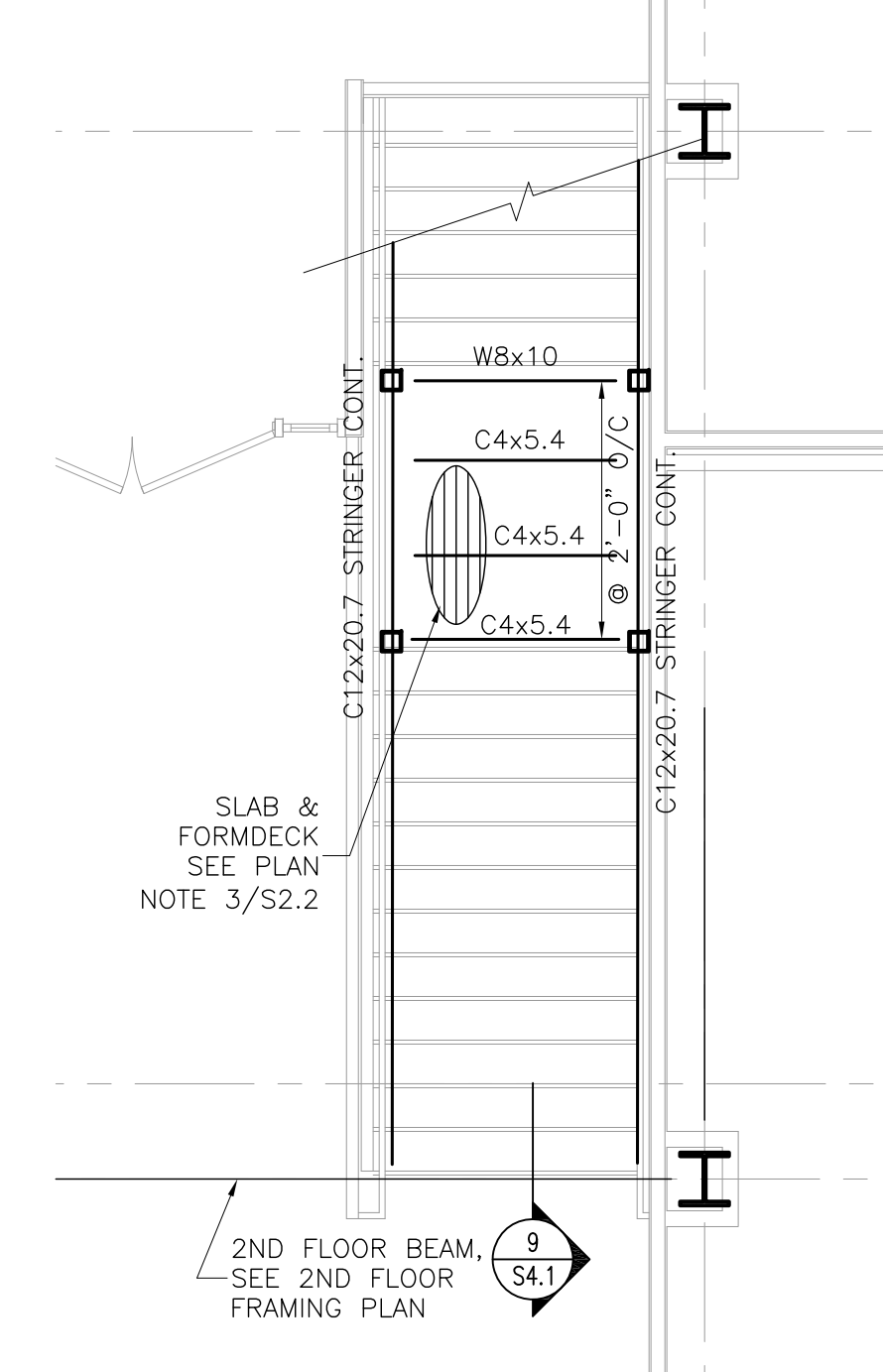
- ALL STAIR FRAMING (STRINGERS, HEADERS, POSTS, ETC.) SHALL BE DESIGNED AND FURNISHED BY STEEL FABRICATOR.

LEGEND:

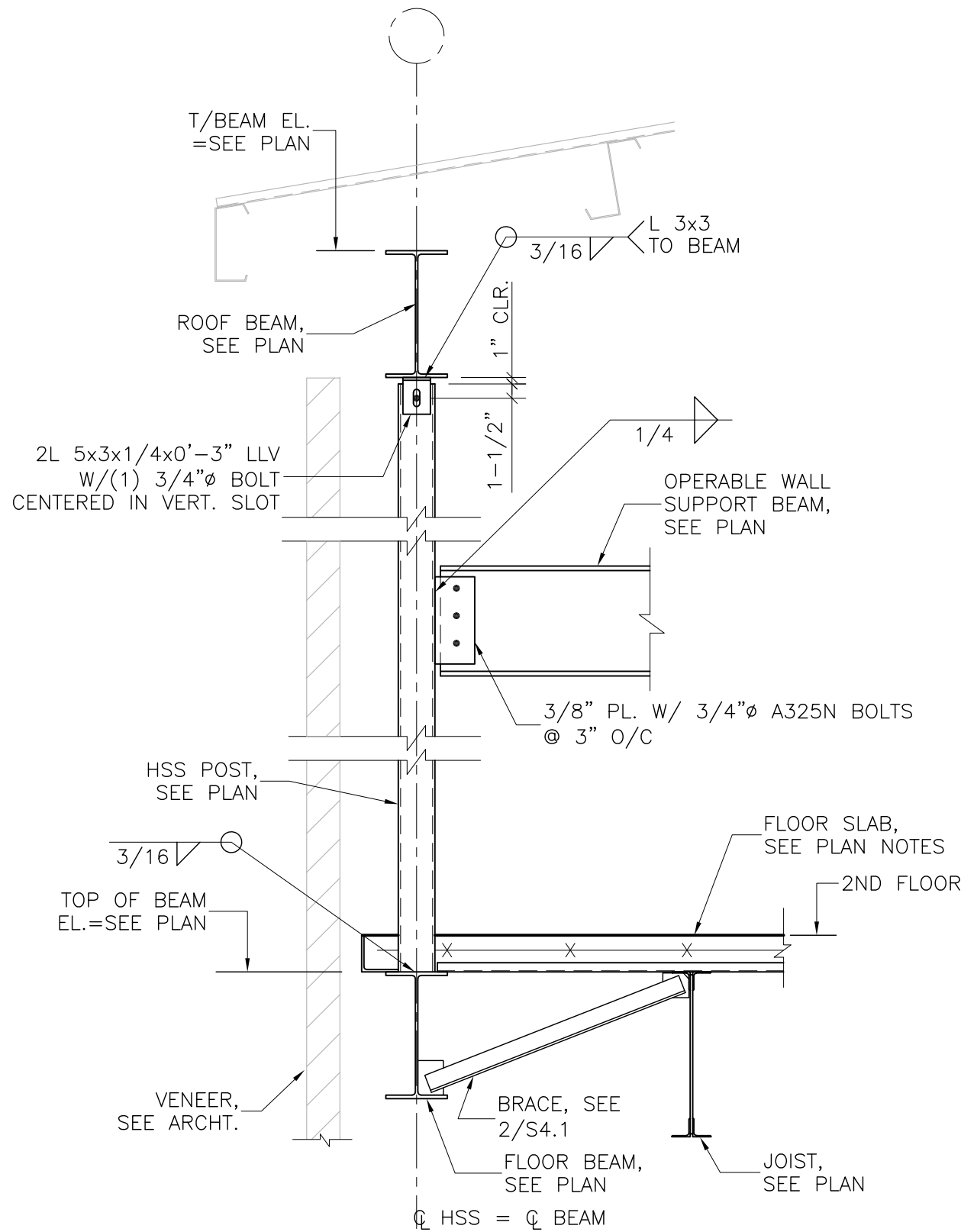
- F.S. FOOTING STEP-SEE 4/S1.0
BASE PLATE TYPE, ALL BASE PLATES ARE 4 U.N. SEE SHEET S1.1 FOR DETAILS
C.J. CONTROL/CONSTRUCTION JOINT SEE SHEET S1.0.



4 HANGER @ CANOPY BEAM CONNECTION
SCALE 3/4" = 1'-0"



2 STAIR #2 INTERMEDIATE LANDING FRAMING PLAN
SCALE 1/4" = 1'-0"



3 POST @ OPERABLE WALL
SCALE 3/4" = 1'-0"

- PLAN NOTES: (CANOPY ROOF)
- SEE SHEET S1.0 FOR GENERAL NOTES AND SCHEDULES
 - SUPERIMPOSED DESIGN LOADS:
ROOF LIVE LOAD = 20 PSF, UNREDUCIBLE
SUPERIMPOSED DEAD LOAD (NOT INCLUDING FRAMING SELFWEIGHT)
PREFABRICATED ROOF SYSTEM = 10 PSF
CEILING/MECHANICAL = 5 PSF
SPRINKLERS = 3 PSF
SEE GENERAL NOTES FOR WIND AND SNOW LOADS.
 - ROOF DECK:
19/32" APA RATED PLYWOOD SHEATHING ON PRE-ENGINEERED LIGHTGAGE STEEL ROOF TRUSS @ 24" O/C. 3 SPAN CONDITION MIN. SUBSTITUTION WITH 1x DECKING WILL NOT BE ALLOWED. THE LEAST DIMENSION OF ROOF DECK SHALL NOT BE LESS THAN 2'-0". ANCHORAGE OF ROOF DECK = SEE 9/S1.2
 - ROOF FRAMING SYSTEM TO BE IN ACCORDANCE WITH "PERFORMANCE SPEC" MEETING PLANS AND PROJECT SPECIFICATIONS. THEREFORE, TRUSS FRAMING SCHEME INDICATED IS SCHEMATIC ONLY. CONTRACTOR IS TO PROVIDE FINAL ROOF SYSTEM DESIGN AND CONSTRUCTION INCLUDING FRAMING SCHEME, DESIGN AND CONSTRUCTION OF ALL TRUSSES AND OTHER PRIMARY AND SECONDARY FRAMING MEMBERS, CONNECTIONS, AND PERMANENT AND TEMPORARY BRACING, BRIDGING, ETC. TO MEET APPLICABLE CODES AND PROJECT DETAILS. ALTERNATE FRAMING SCHEMES ARE SUBJECT TO A/E APPROVAL AND WILL NOT BE PERMITTED IF THEY ALTER GRAVITY OR LATERAL LOAD PATHS TO FOUNDATIONS OR OTHER SUPPORTING MEMBERS.

- PLAN NOTES: (SECOND FLOOR FRAMING)
- SEE SHEET S1.0 THRU S1.3 FOR GENERAL NOTES, SCHEDULES & TYPICAL DETAILS
 - SUPERIMPOSED DESIGN LOADS: (JOIST WEIGHT NOT INCLUDED)
LIVE LOADS =
CORRIDORS = 80 psf
CLASSROOMS = 40 psf
STORAGE & MECH. = 125 psf
LOBBY, STAIRS = 100 PSF
ASSEMBLY AREAS, MEETING ROOMS = 100 PSF
OFFICES = 50 PSF
DEAD LOADS =
4" NON-COMPOSITE SLAB = 44 PSF
CEILING/MECHANICAL = 10 PSF
PARTITIONS = 20 PSF
SPRINKLERS = 3 PSF
FRAMING = ACTUAL
 - TYP. FLOOR: 4" NON-COMPOSITE SLAB U.N. 15/16" 26 GA. GALVANIZED FORM DECK (EQUAL TO VULCRAFT 1.0C26 OR BETTER)
W/ 3" 1/16" MIN. SOLID THICKNESS N.W. CONCRETE
FILL W/ W.W.F. - 6x6-W2.9AW2.9 (PLACED 1" BELOW TOP OF SLAB) ON JOISTS SPACED AT 2'-6" O/C (MAX.)
 - PROVIDE JOIST BRIDGING IN ACCORDANCE WITH S.J.I. REQUIREMENTS.
 - JOIST BEARING ELEVATION = +14'-5 1/2" U.N. @ FLOOR
 - JOISTS SUPPORTING CONCENTRATED OR NON-UNIFORM LOADS SHALL BE "SP" JOISTS IN LIEU OF THE JOIST DESIGNATION SHOWN ON PLANS. LOADS FROM PIPES (6" DIA OR GREATER) SUPPORTED FROM JOISTS ARE NOT INCLUDED IN THE JOIST DESIGNATION SHOWN ON FRAMING PLANS. JOIST DESIGNER SHALL DESIGN JOISTS TO SUPPORT PIPE LOADS, IN ADDITION TO THE GRAVITY LOADS NOTED ABOVE. COORDINATE PIPE HANGER LOCATIONS AND WEIGHTS WITH RESPECTIVE VENDORS.
 - DECK TO JOIST ANCHORAGE, SEE 9/S1.2
 - JOIST DESIGNED FOR ADDITIONAL WT. OF AHU & PAD +40 PSF LIVE LOAD OR 125 PSF LL, WHICHEVER RESULTS IN GREATER STRESS.

SECOND FLOOR FRAMING PLAN
SCALE 1/8" = 1'-0"

- LEGEND:
- MOMENT RESISTING CONNECTION
SEE S1.3. DENOTES SEISMIC FORCE RESISTING SYSTEM, SEE PROTECTED ZONE ON 2/S1.3
 - COLUMN STOPS @ THIS LEVEL
SEE 8/S5.1
 - T1 PRE-ENGINEERED LIGHT GAGE TRUSS, SEE S5.1
 - ST1 SHEAR TRUSS, SEE S5.1
 - C=3/4" UPWARD CAMBER @ MID SPAN
 - (C) CANTILEVER, SEE 6/S1.2
 - BLOCKING TRUSS, SEE S5.1
 - AXIAL CONNECTION, SEE 6/S1.3
 - EP EMBEDDED PLATE, SEE 11/S4.1

TOM ROBISON & ASSOCIATES, INC.
STRUCTURAL ENGINEERS
1716 KIRBY PARKWAY - SUITE 201
MEMPHIS, TENNESSEE 38120
901.754.4411
sarfrax_k@bellsouth.net

PROJECT NO. 160103
DATE: 05-21-16
DRAWN BY: RC

REVISION:
DATE:

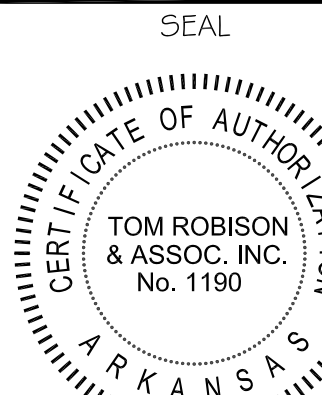
NOTICE
THIS DRAWING IS THE INSTRUMENT OF SERVICE AND THE PROPERTY OF MATT SILAS ARCHITECT. IT IS TO BE USED SOLELY FOR THIS PROJECT.
COPYRIGHT © 2018

MSA
MATT SILAS ARCHITECT
Jonesboro, Arkansas 72401
212 East Washington Ave.
Tel: (870) 268-0500 Fax: (870) 268-0501
Website: mattsilasarchitect.com Email: mattsilas@sbeglobal.net

CENTRAL BAPTIST CHURCH
ADMINISTRATION WING ADDITION

3707 HARRISBURG ROAD
JONESBORO, ARKANSAS 72404

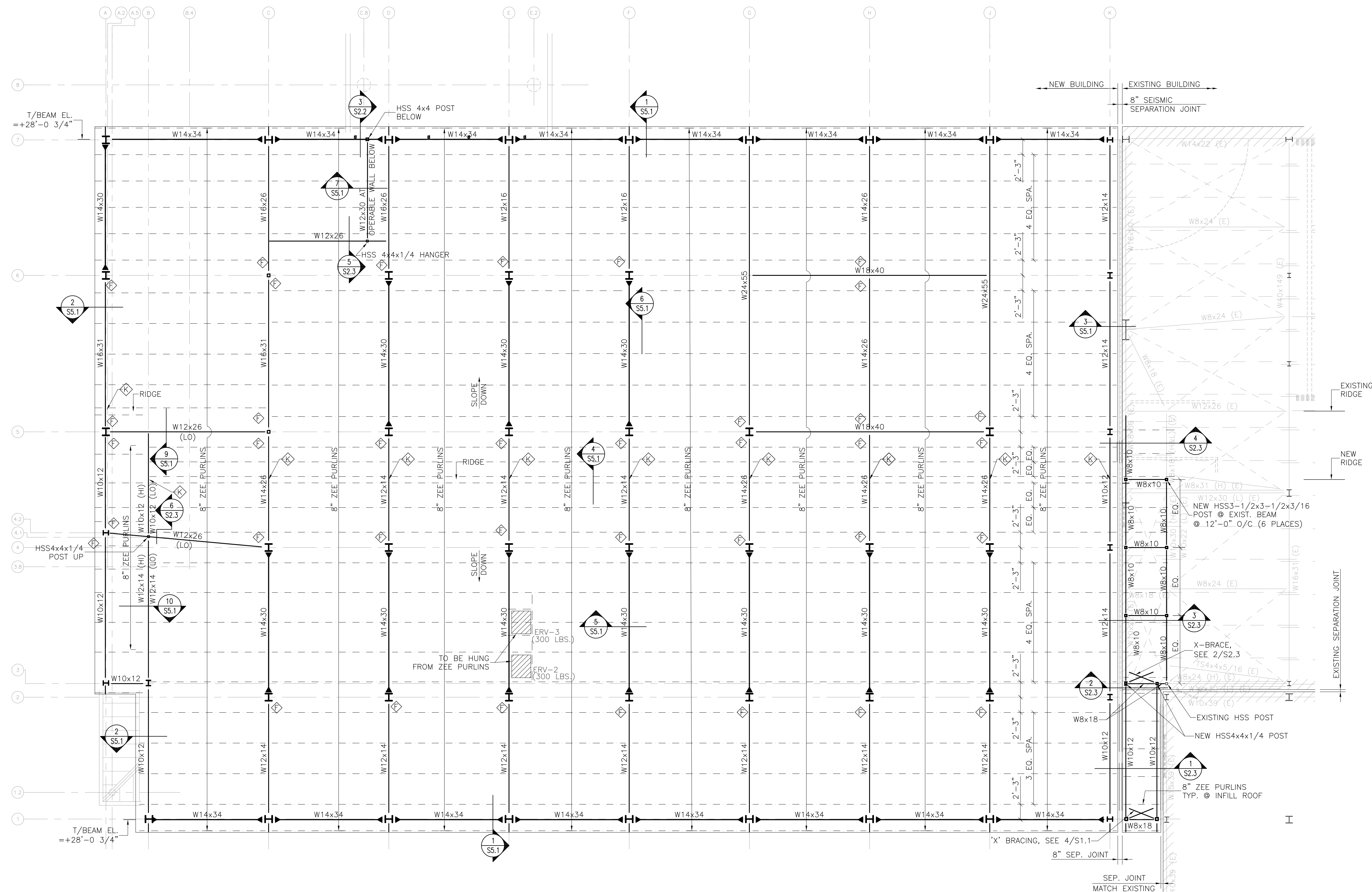
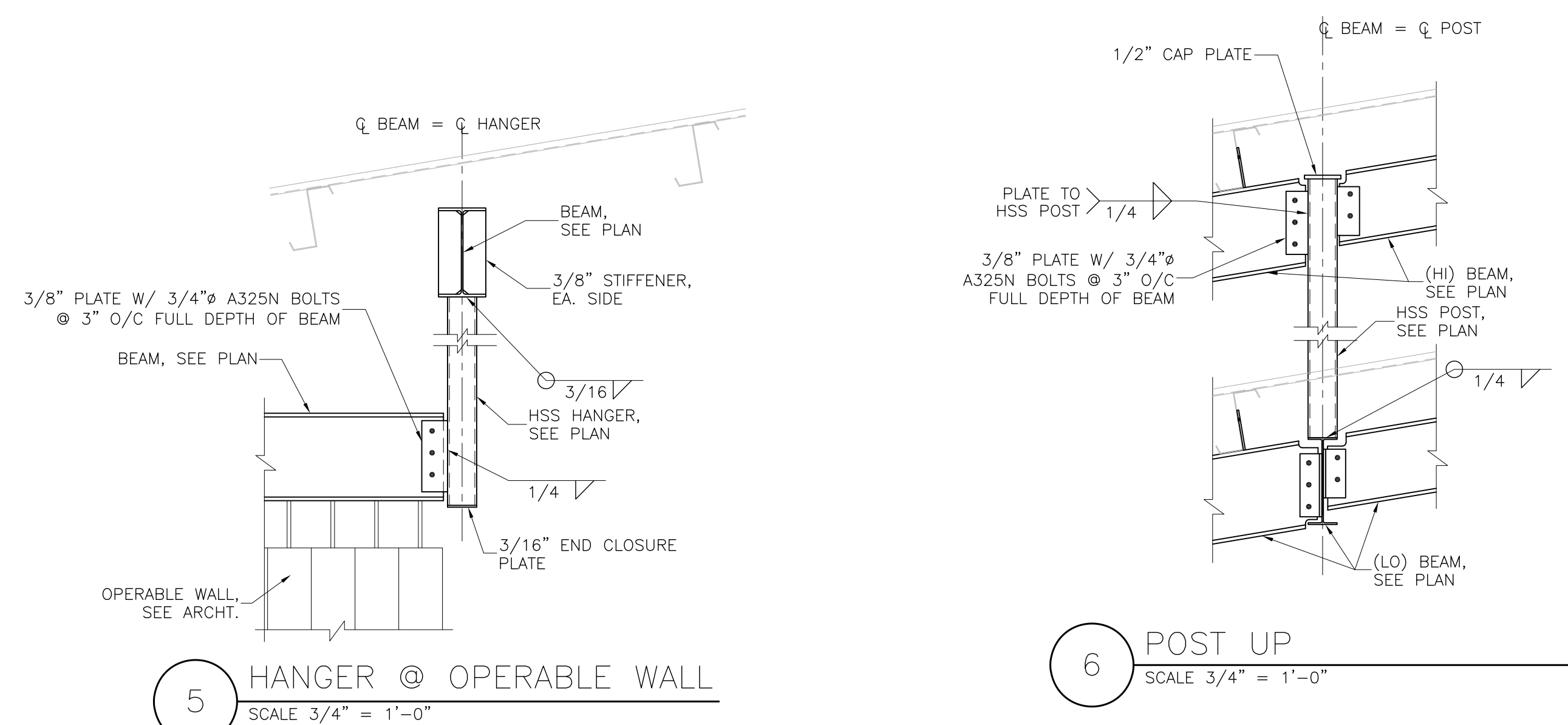
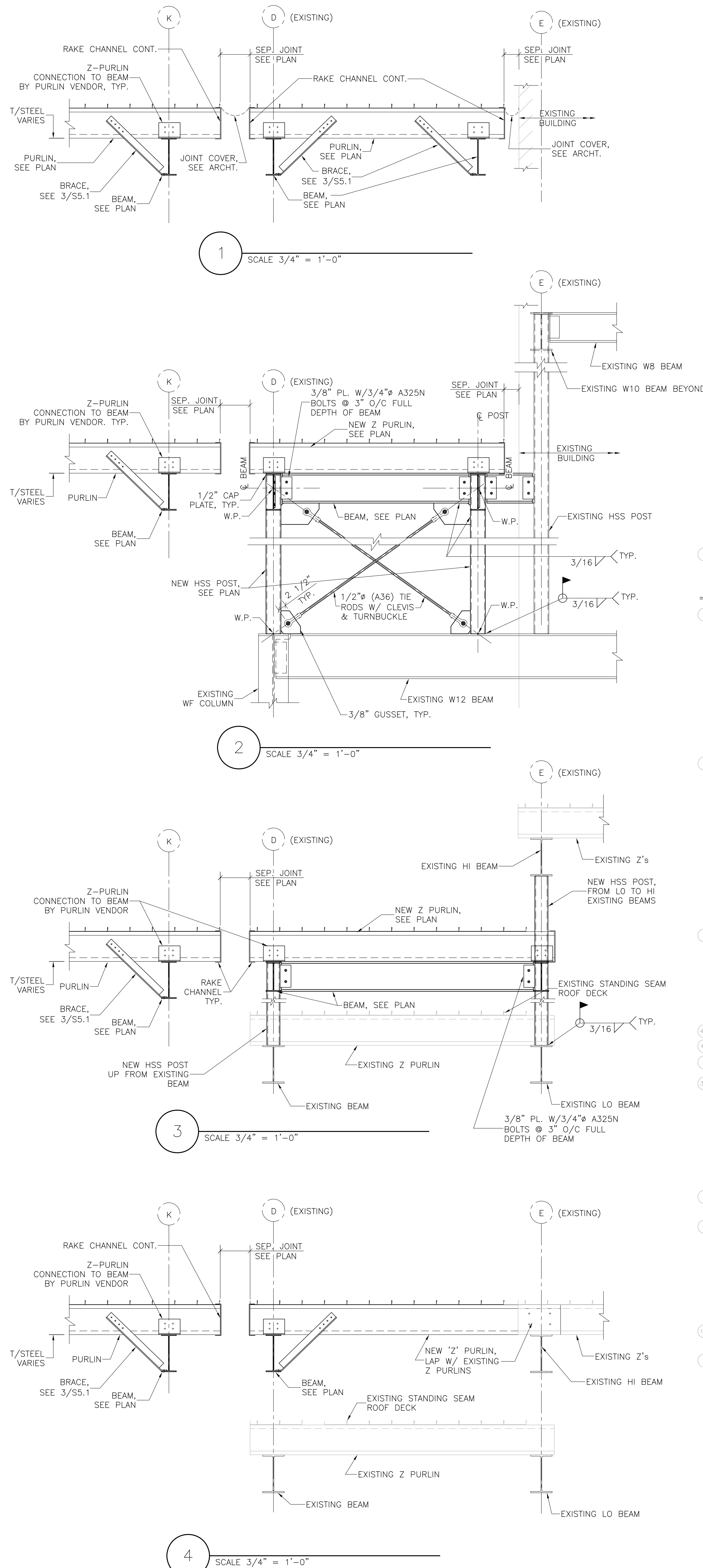
centralbaptist



SECOND FLOOR FRAMING PLAN

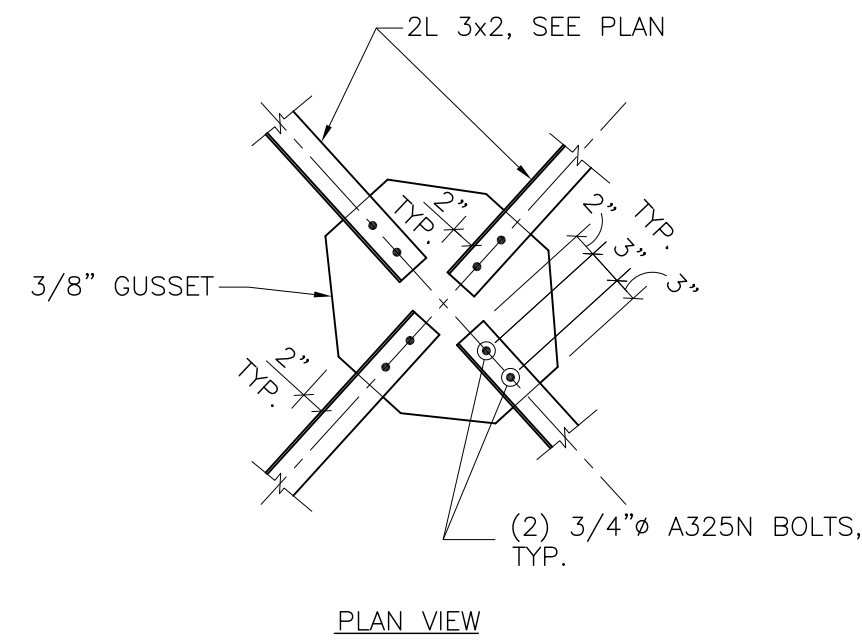
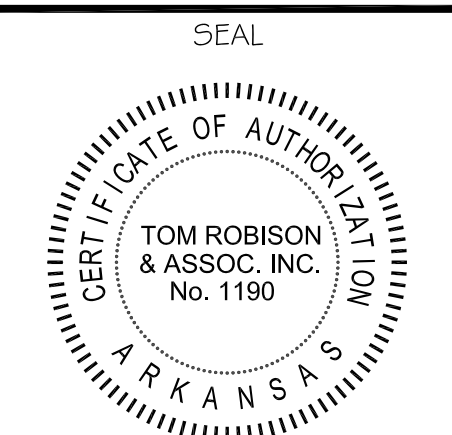
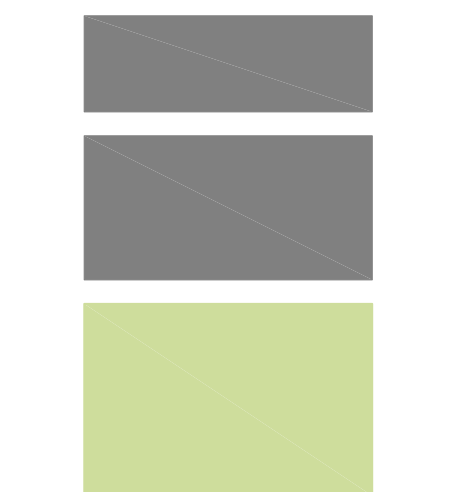
S2.2

SHEET NO.

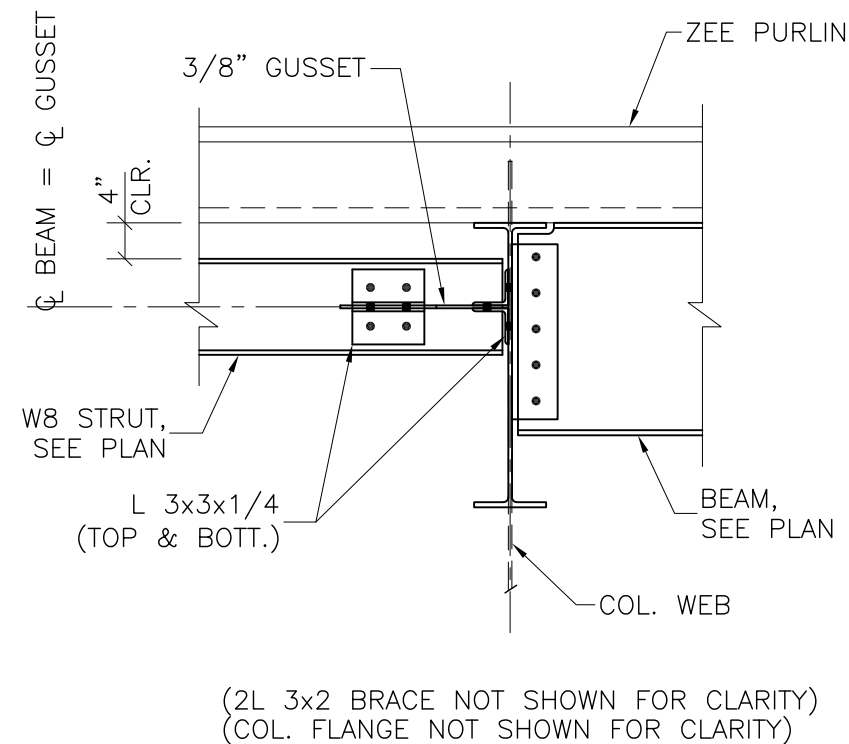


- PLAN NOTES:
- SEE SHEET S1.0 THRU S1.2 FOR GENERAL NOTES, SCHEDULES AND TYPICAL DETAILS.
 - SUPERIMPOSED DESIGN LOADS: (EXCLUDING TRUSS/JOIST SELF WEIGHTS)
ROOF LIVE LOAD = 20 PSF
DECK LOAD = 2 PSF
CEILING/MECHANICAL = 10 PSF
ROOFING/INSULATION = 5 PSF
SPRINKLERS = 3 PSF
WIND UPLIFT = SEE GENERAL NOTES ON S1.0
3. DECK BEARING ELEVATION VARIES = SEE ARCHT.
4. STANDING SEAM DECK: BATTENLOK HS 24 GA. (FY=50 KSI) BY MBCI

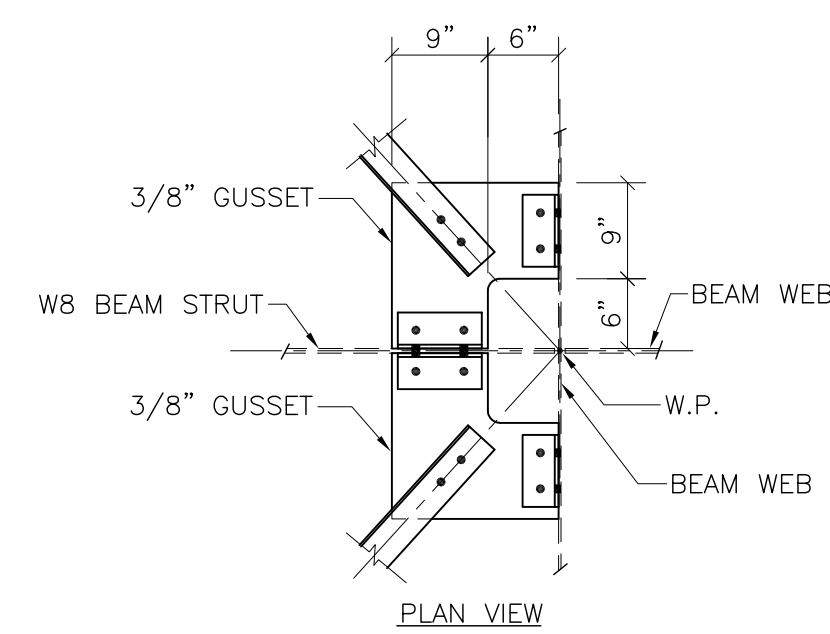
- LEGEND:
- MOMENT RESISTING CONNECTION, SEE S1.3
SEE S1.3. DENOTES SEISMIC FORCE RESISTING SYSTEM, SEE 2/S1.3 FOR PROTECTED ZONE.
 - CRANKED BEAM CONNECTION
SEE 11/S1.2
 - 8" ZEE PURLIN TO BE DESIGNED BY VENDOR.
SEE GENERAL NOTES ON S1.0
 - CANTILEVER
 - AXIAL CONNECTION, SEE 6/S1.3



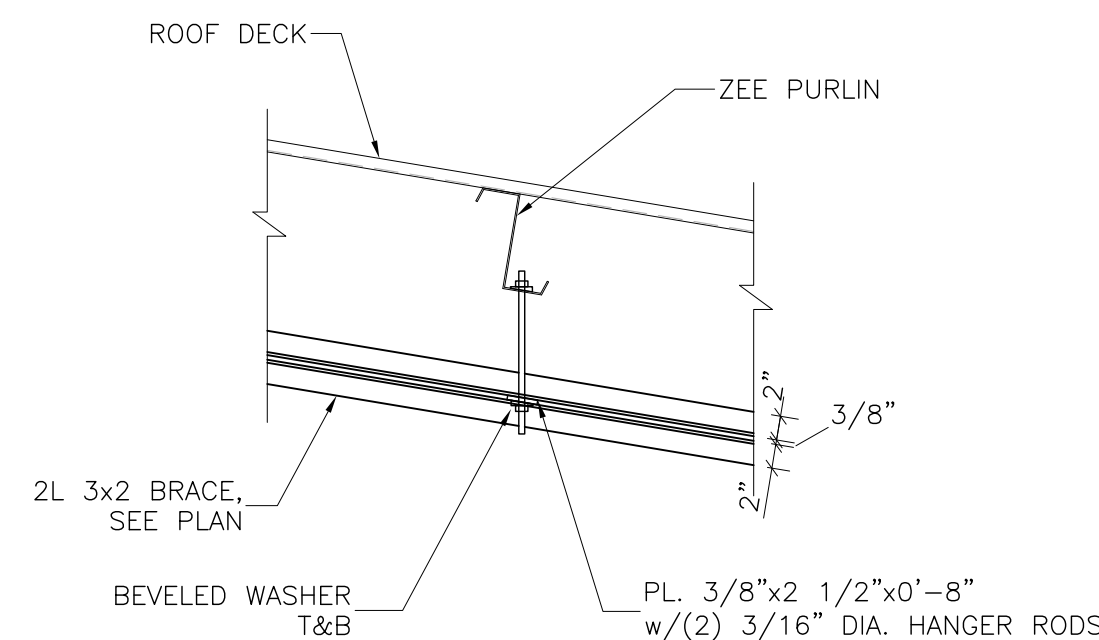
4 TYP. HORIZ. BRACE INTERSECTION
SCALE 3/4" = 1'-0"



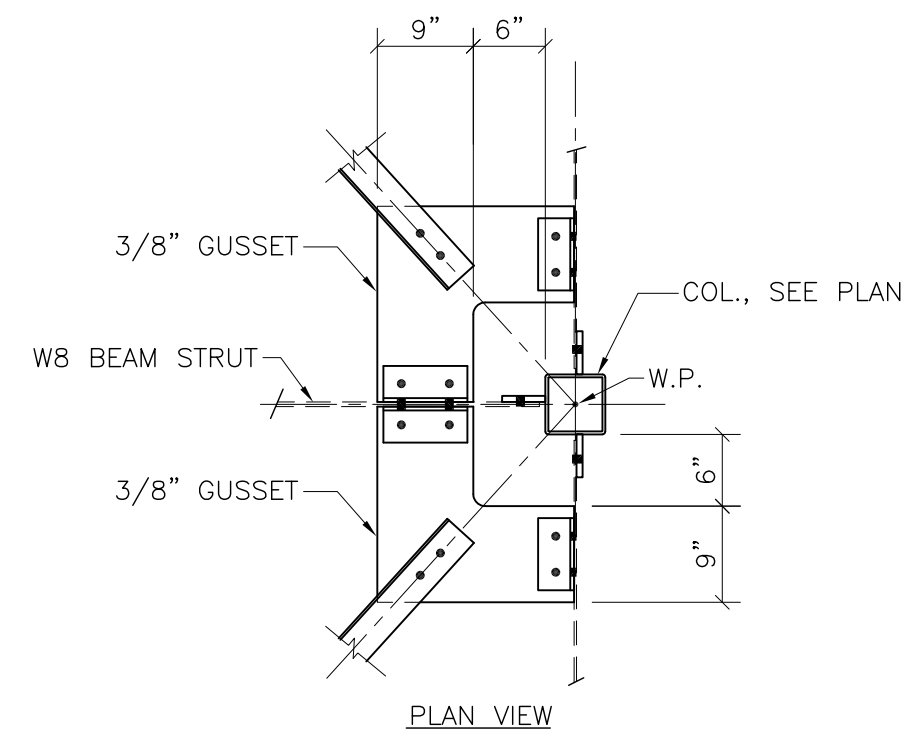
5 TYP. SECTION @ HORIZ. BRACE
SCALE 3/4" = 1'-0"



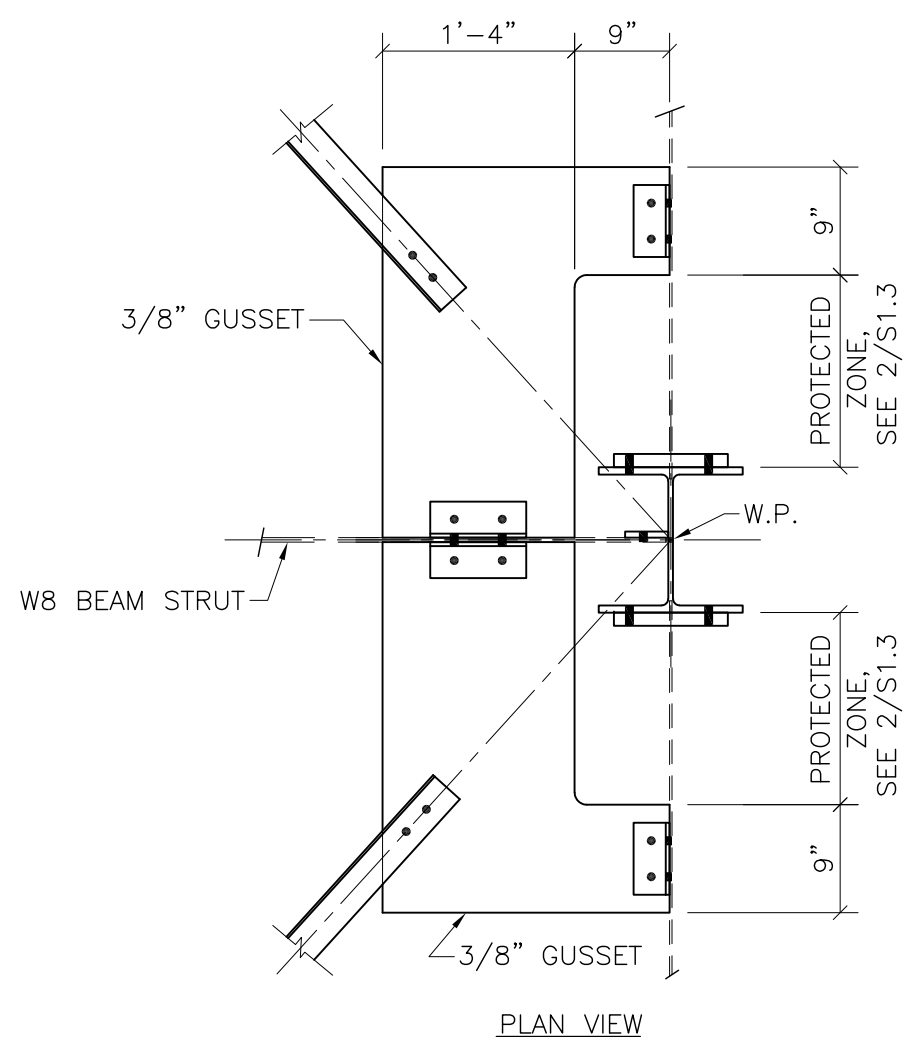
6 TYP. HORIZ. BRACE AT INTERIOR BEAM
SCALE 3/4" = 1'-0"



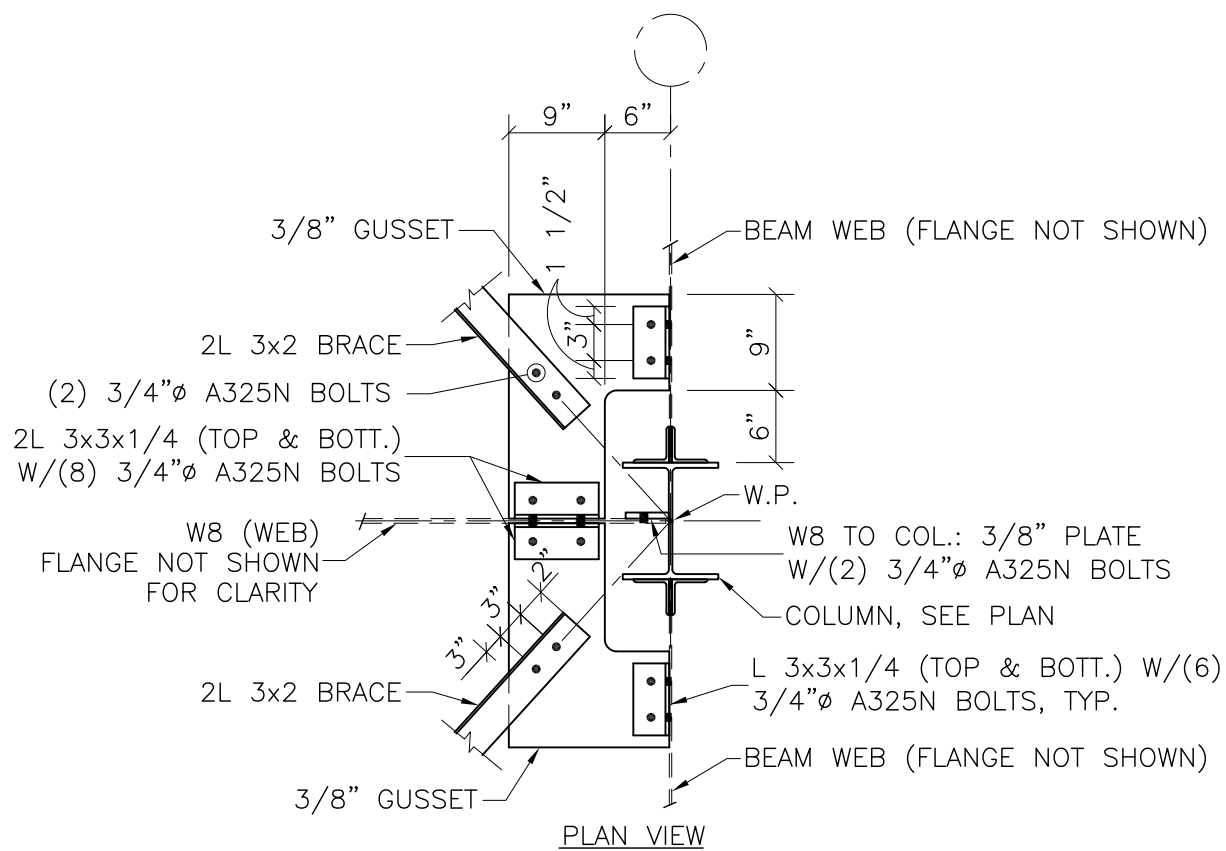
7 TYP. HORIZ. BRACE SUPPORT
SCALE 3/4" = 1'-0"



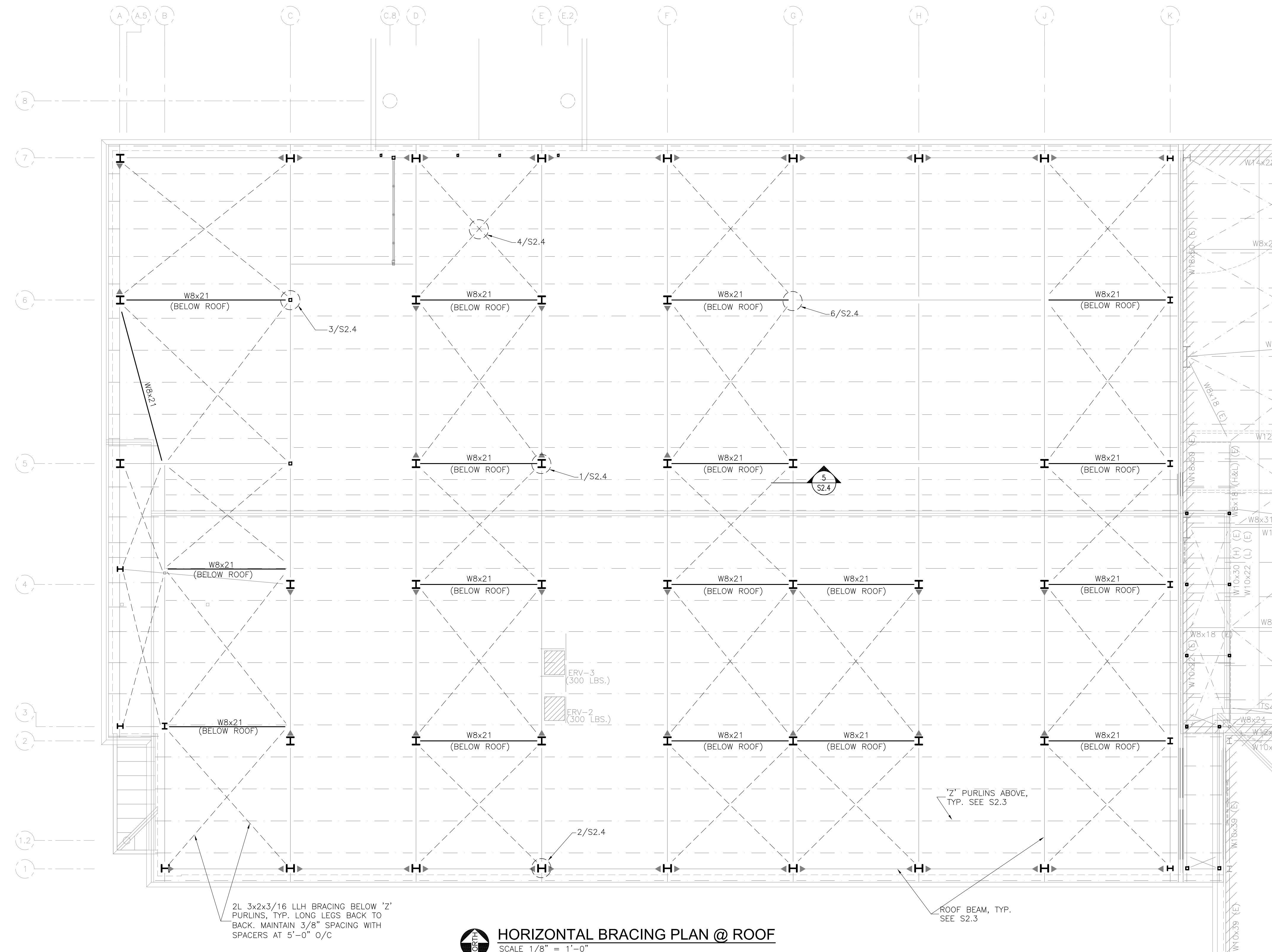
3 TYP. HORIZ. BRACE AT HSS COL.
SCALE 3/4" = 1'-0"



2 TYP. HORIZ. BRACE AT W12 COL.
SCALE 3/4" = 1'-0"

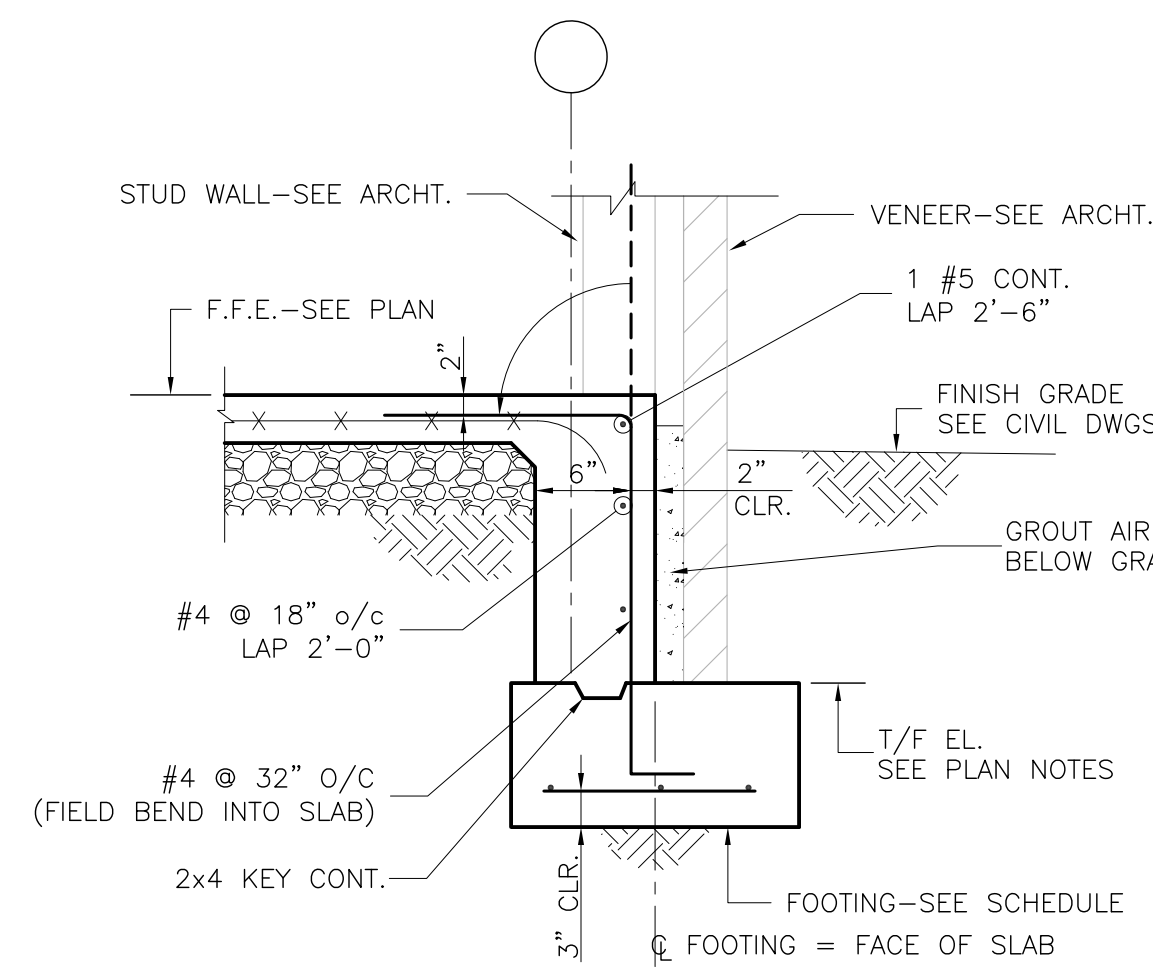
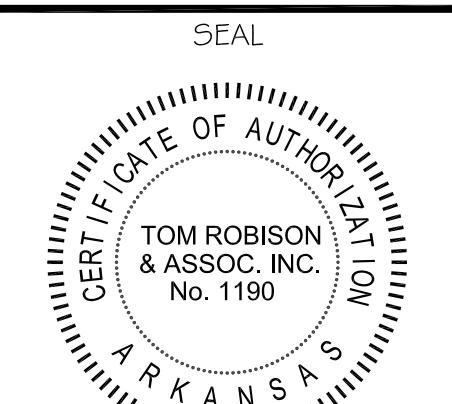
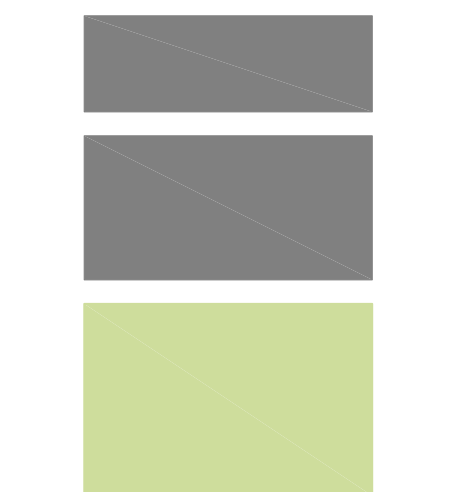


1 TYP. HORIZ. BRACE AT ALL COLS., EXCEPT W12
SCALE 3/4" = 1'-0"

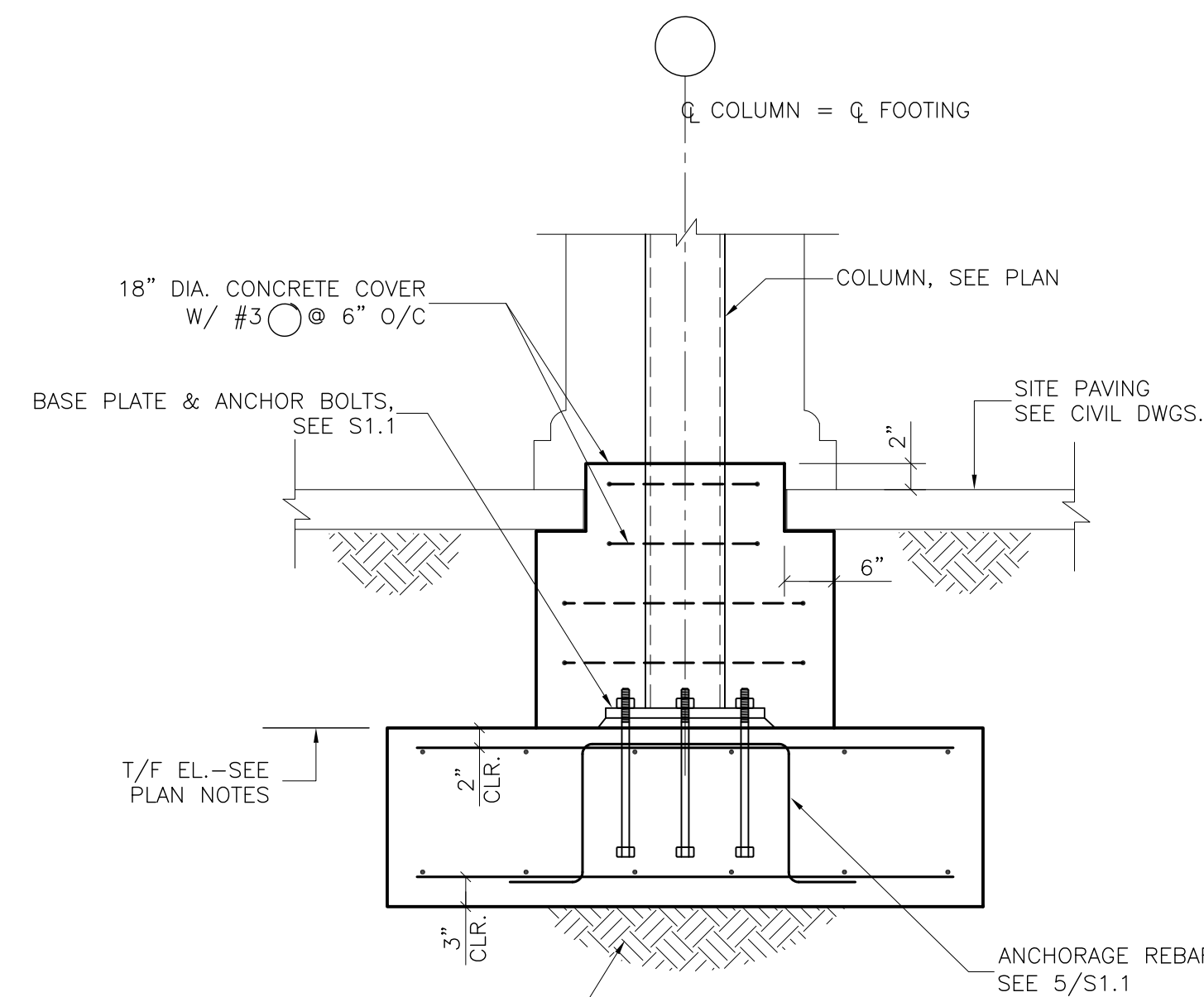


HORIZONTAL BRACING PLAN @ ROOF
SCALE 1/8" = 1'-0"
PLAN NOTES:

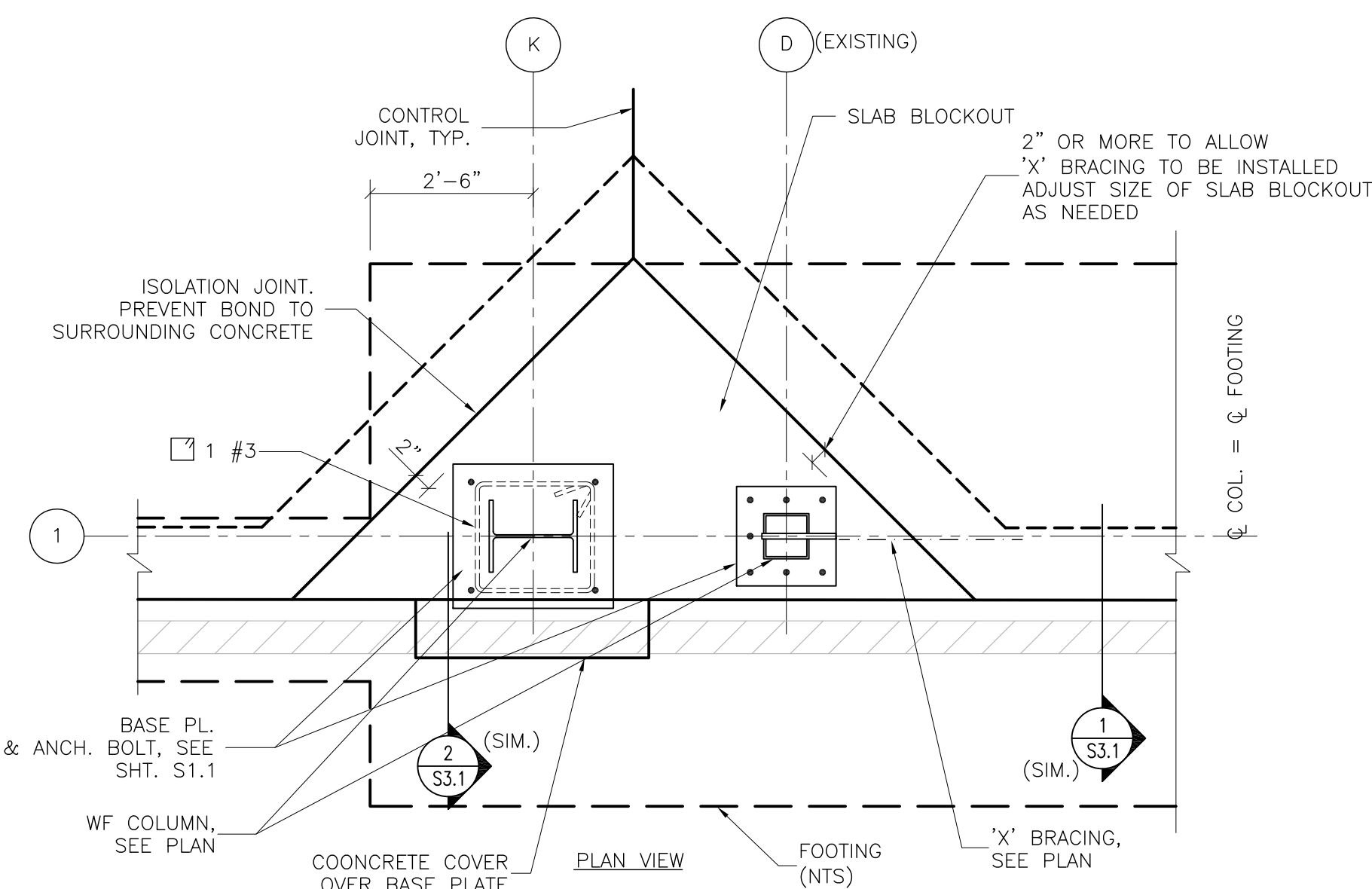
- SEE SHEET S1.0 THRU S1.3 FOR GENERAL NOTES, SCHEDULES AND TYPICAL DETAILS.
- TOP OF W8x21 STRUT @ 4" BELOW 'Z' PURLIN.
- 2L 3x2 BRACING IS CENTERED ON W8 STRUT. SLOPE BRACING WITH ROOF SLOPE, EXCEPT FLAT WHERE CROSSING RIDGE.
- SUPPORT 2L3x2 BRACE @ EACH PURLIN, SEE 7/S2.4.
- PROVIDE BENT PLATE OR BENT ANGLE IN LIEU OF CONNECTION ANGLES SHOWN IN SECTIONS, IF NEEDED FOR FIT.



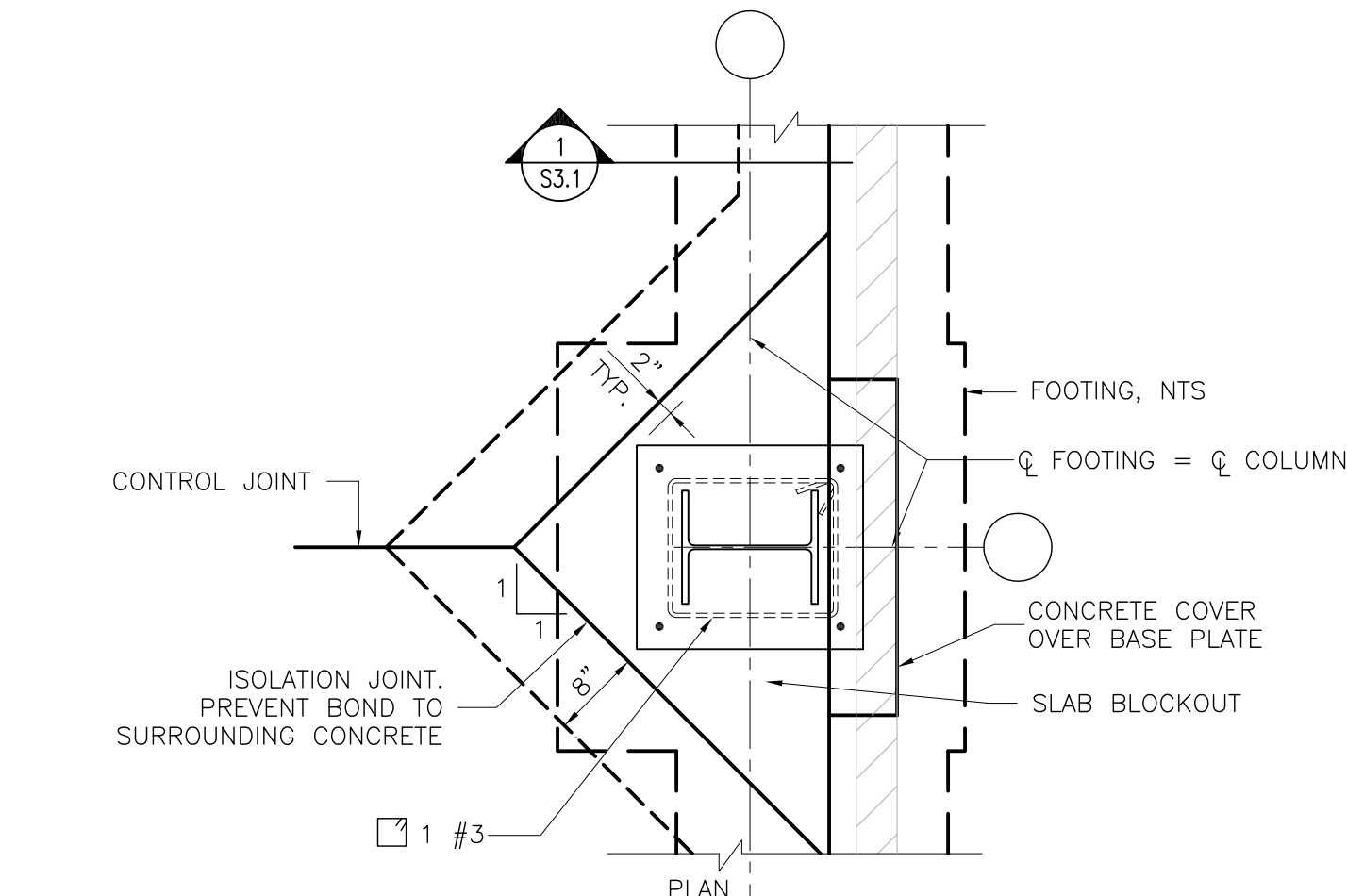
1 TYP. SLAB EXTERIOR
SCALE 3/4" = 1'-0"



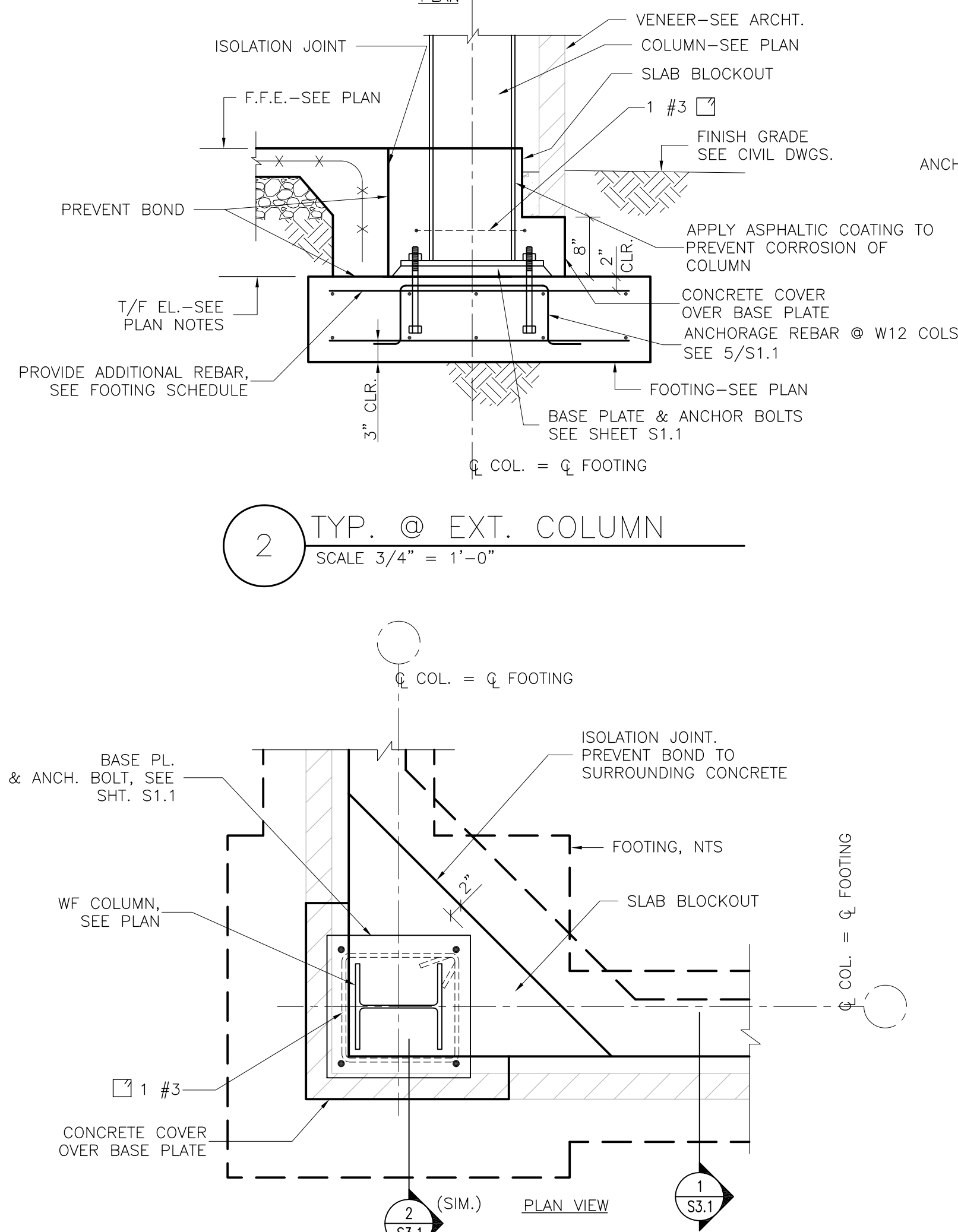
5 TYP. @ CANOPY COLUMN
SCALE 3/4" = 1'-0"



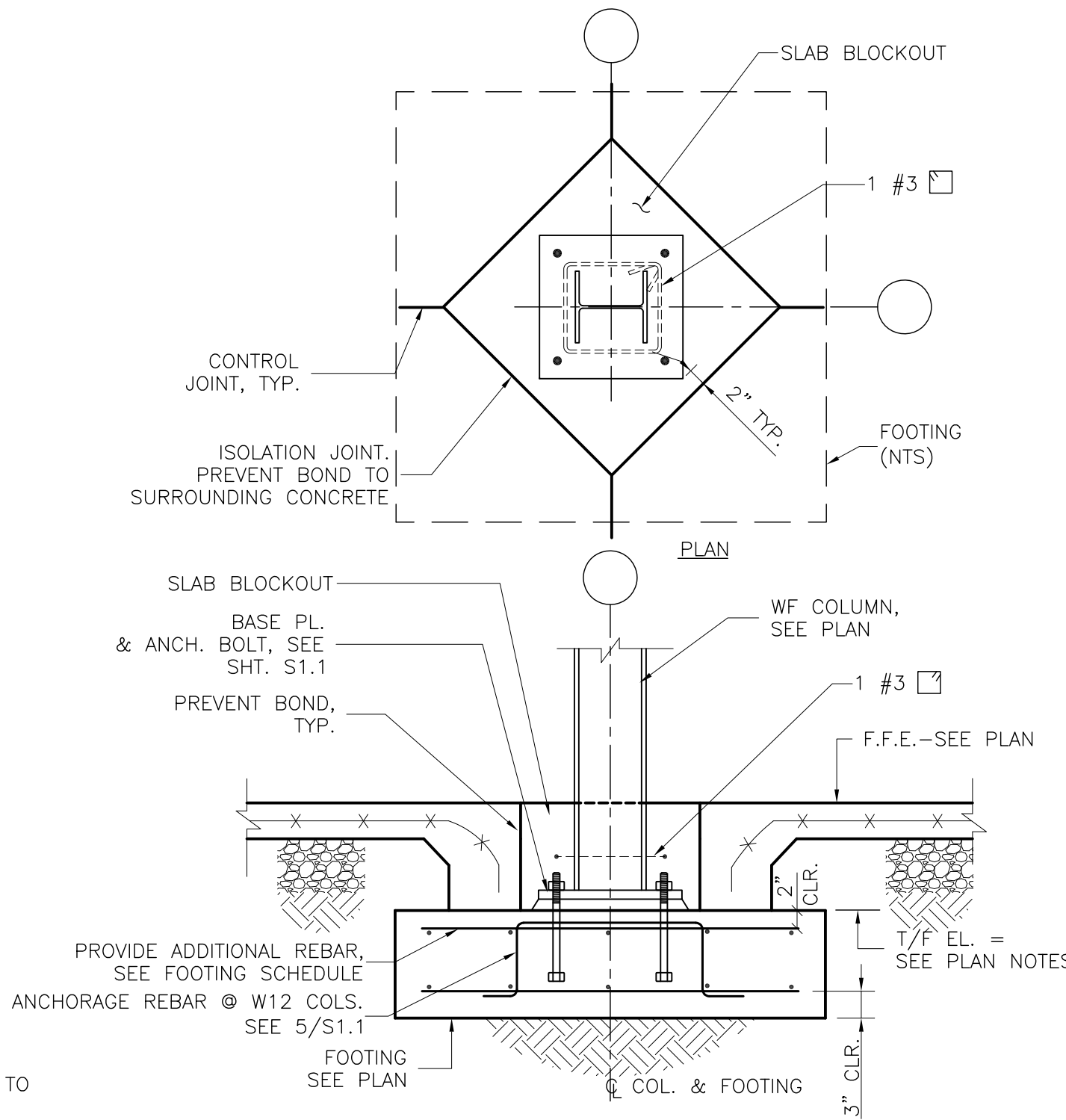
9 SCALE 3/4" = 1'-0"



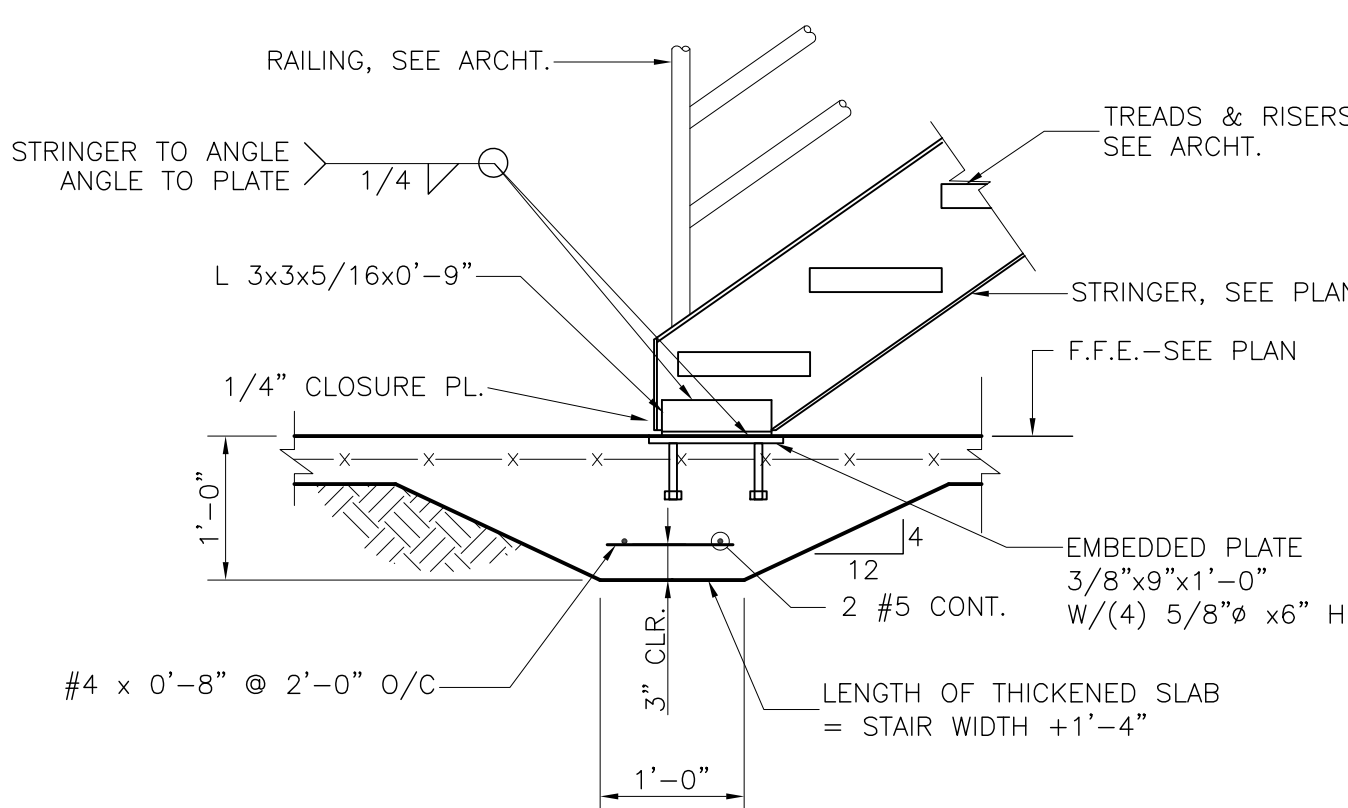
2 TYP. @ EXT. COLUMN
SCALE 3/4" = 1'-0"



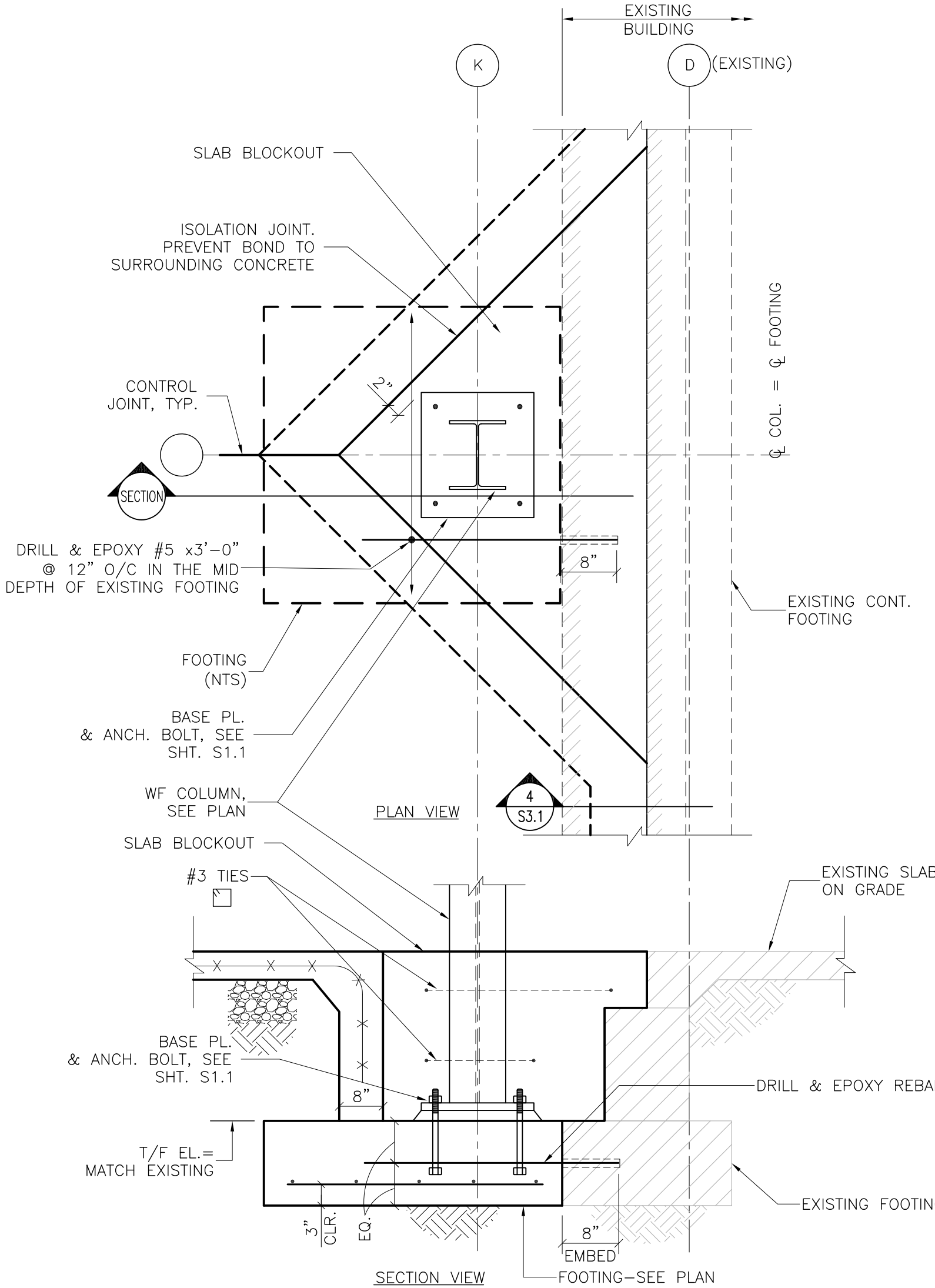
6 TYP. @ CORNER COLUMN
SCALE 3/4" = 1'-0"



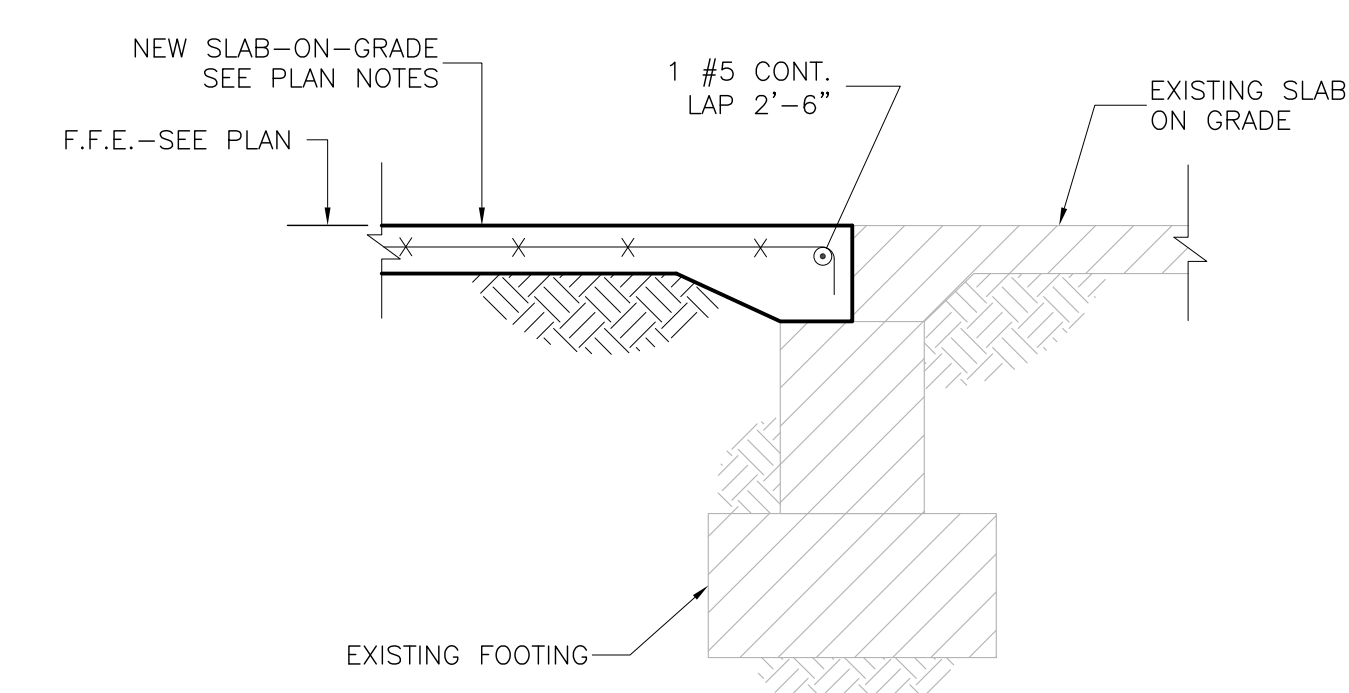
3 TYP. @ INTERIOR COLUMN
SCALE 3/4" = 1'-0"



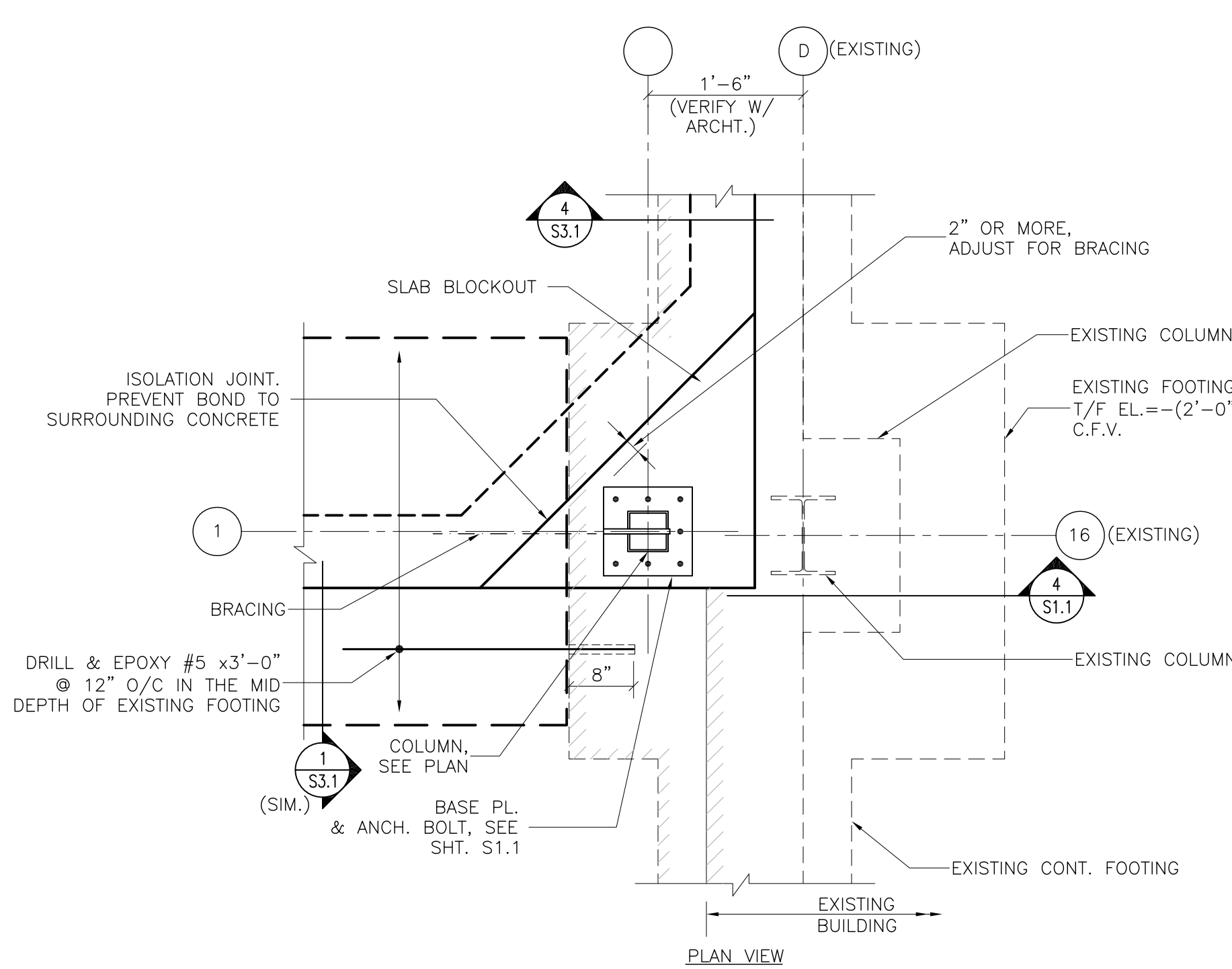
7 TYP. STRINGER TO SLAB ON GRADE
SCALE 3/4" = 1'-0"



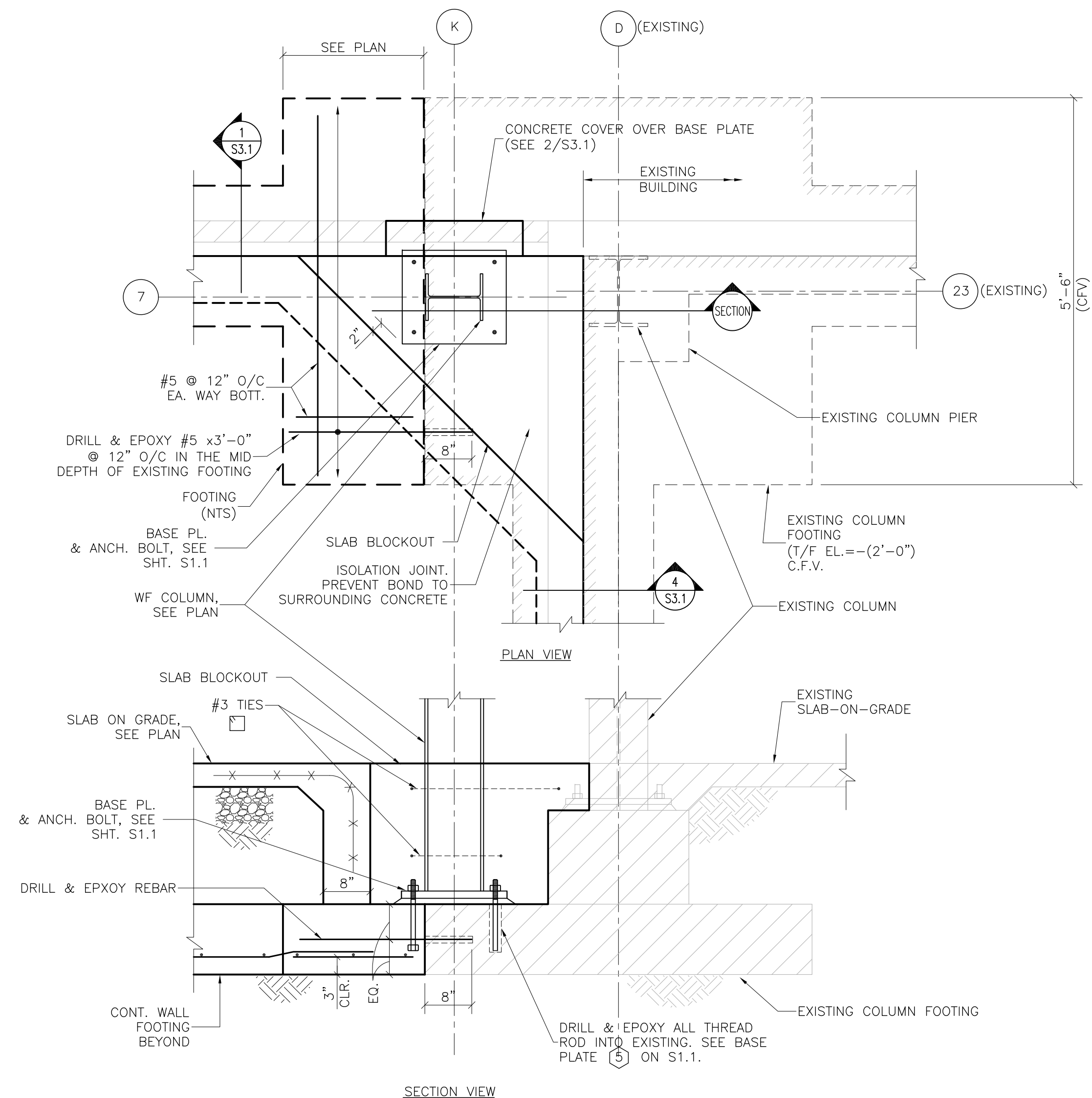
10 TYP. COLUMN NEAR EXIST. BUILDING
SCALE 3/4" = 1'-0"



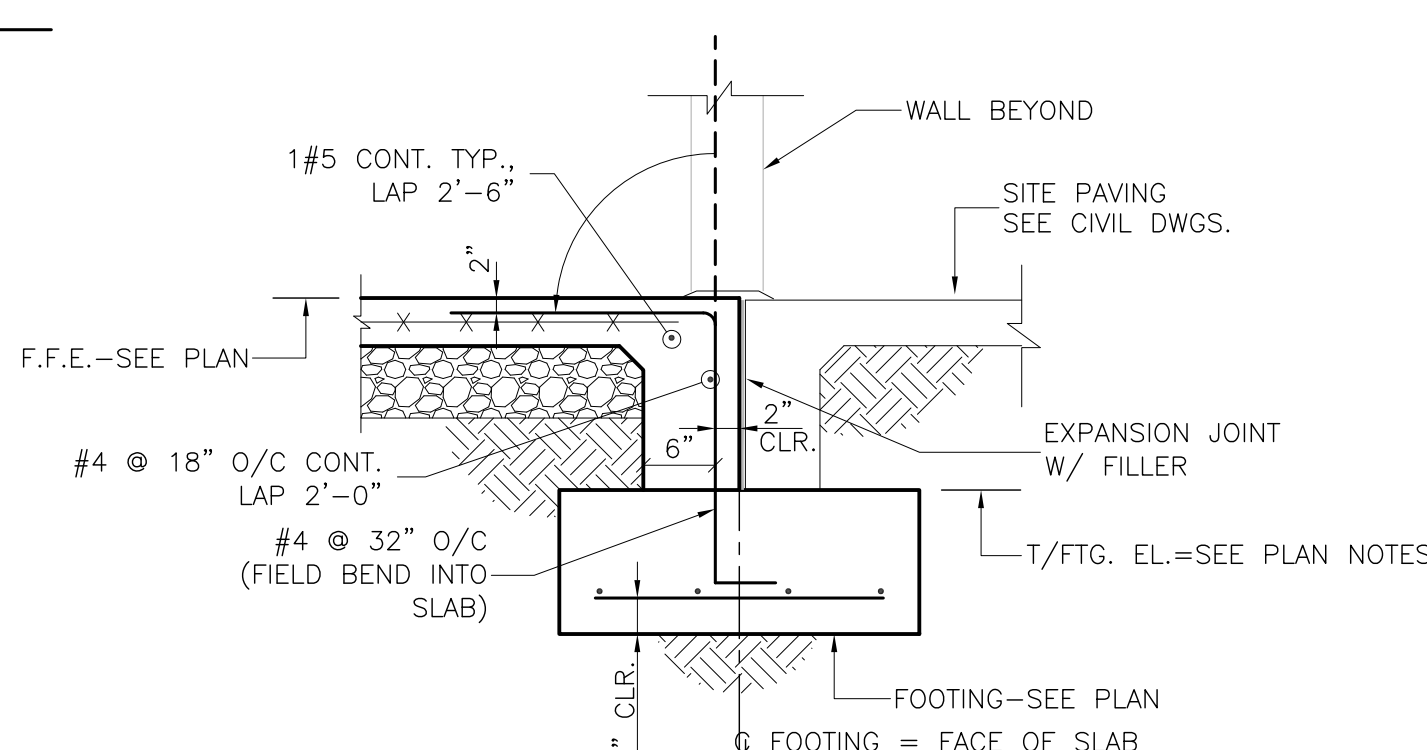
4 TYPICAL SLAB EDGE @ EXISTING BUILDING
SCALE 3/4" = 1'-0"



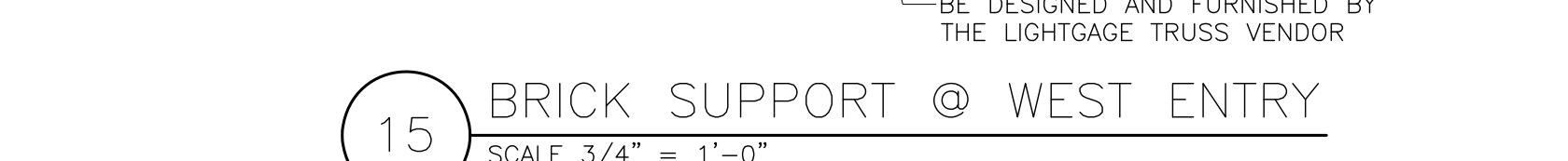
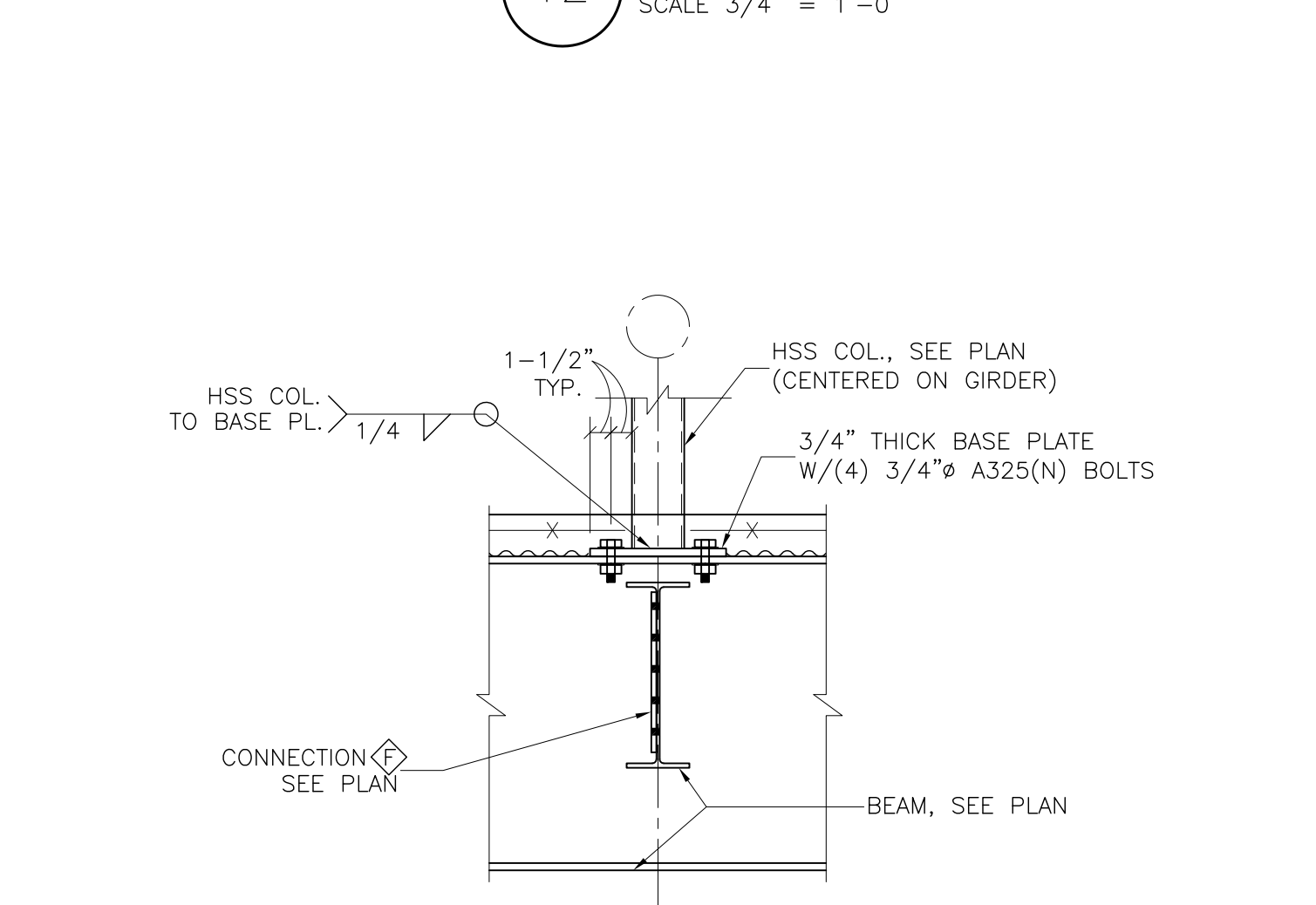
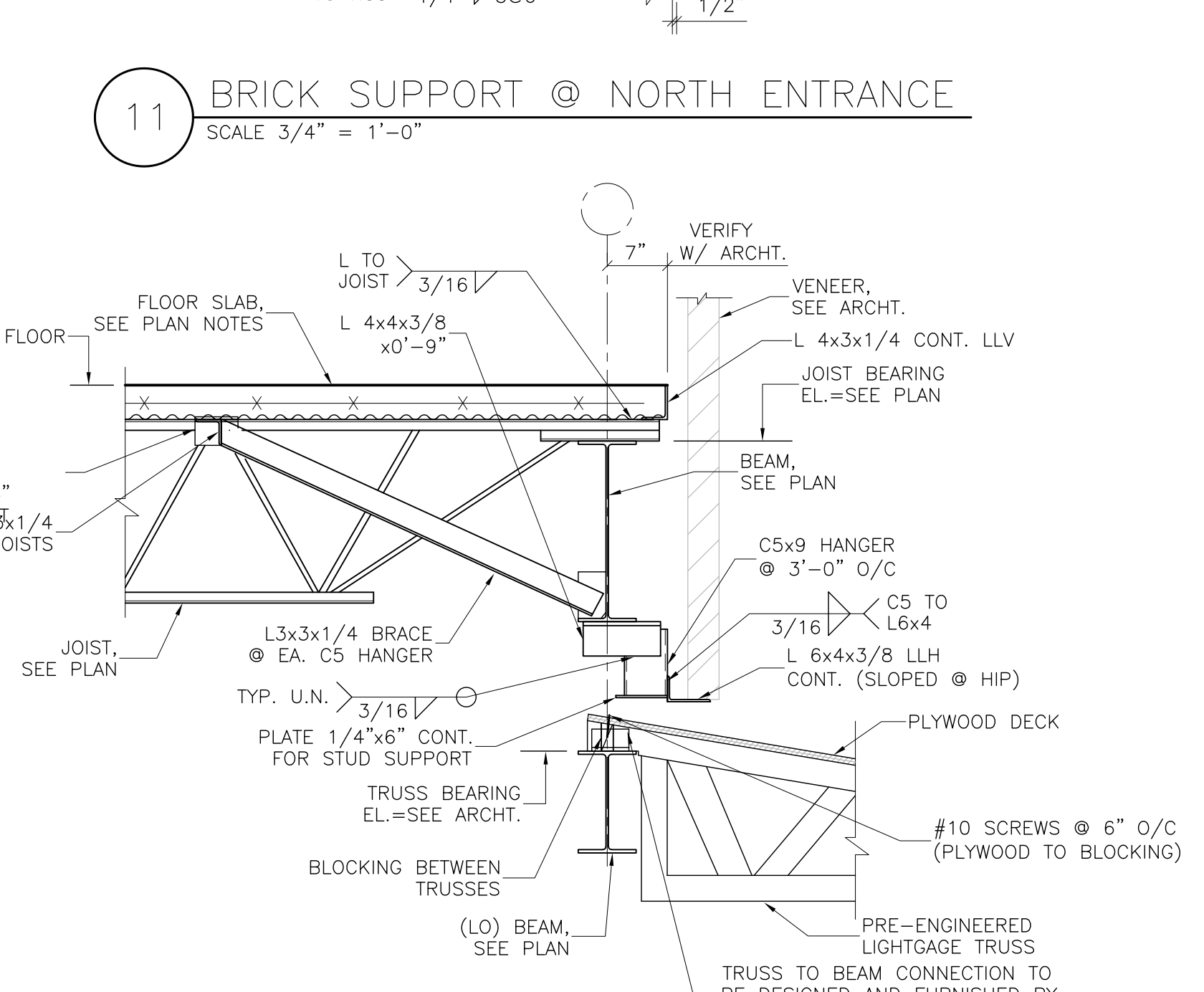
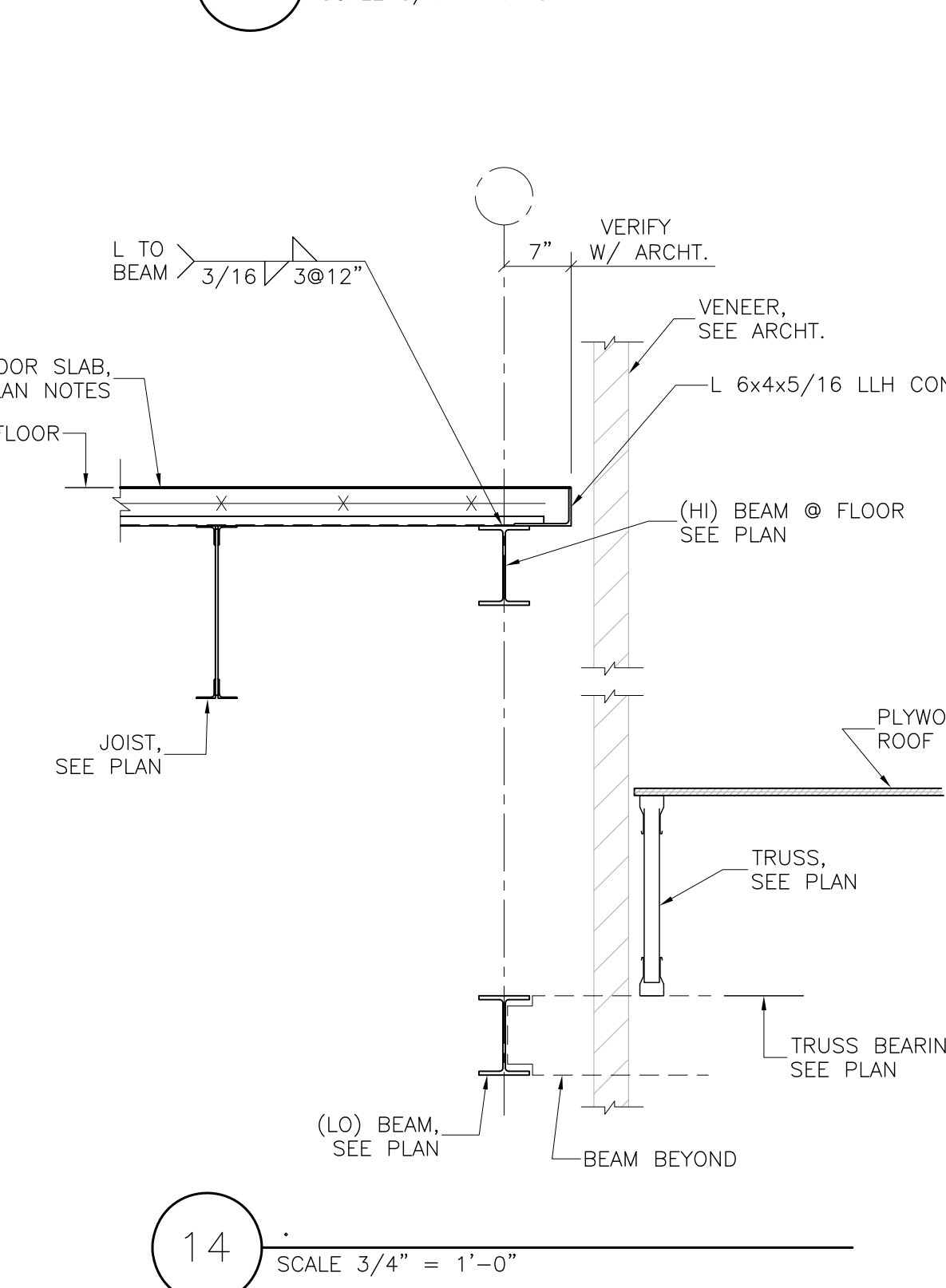
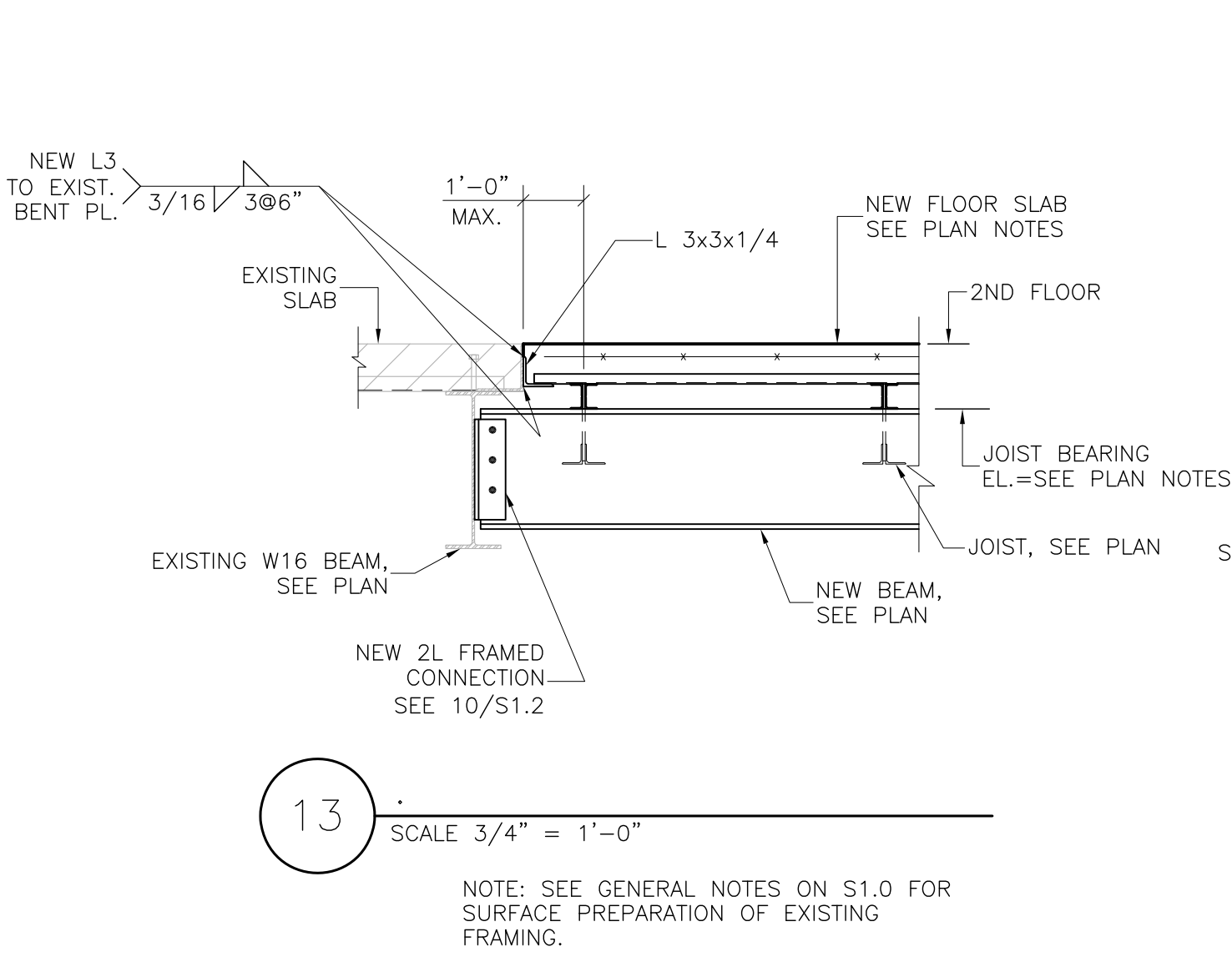
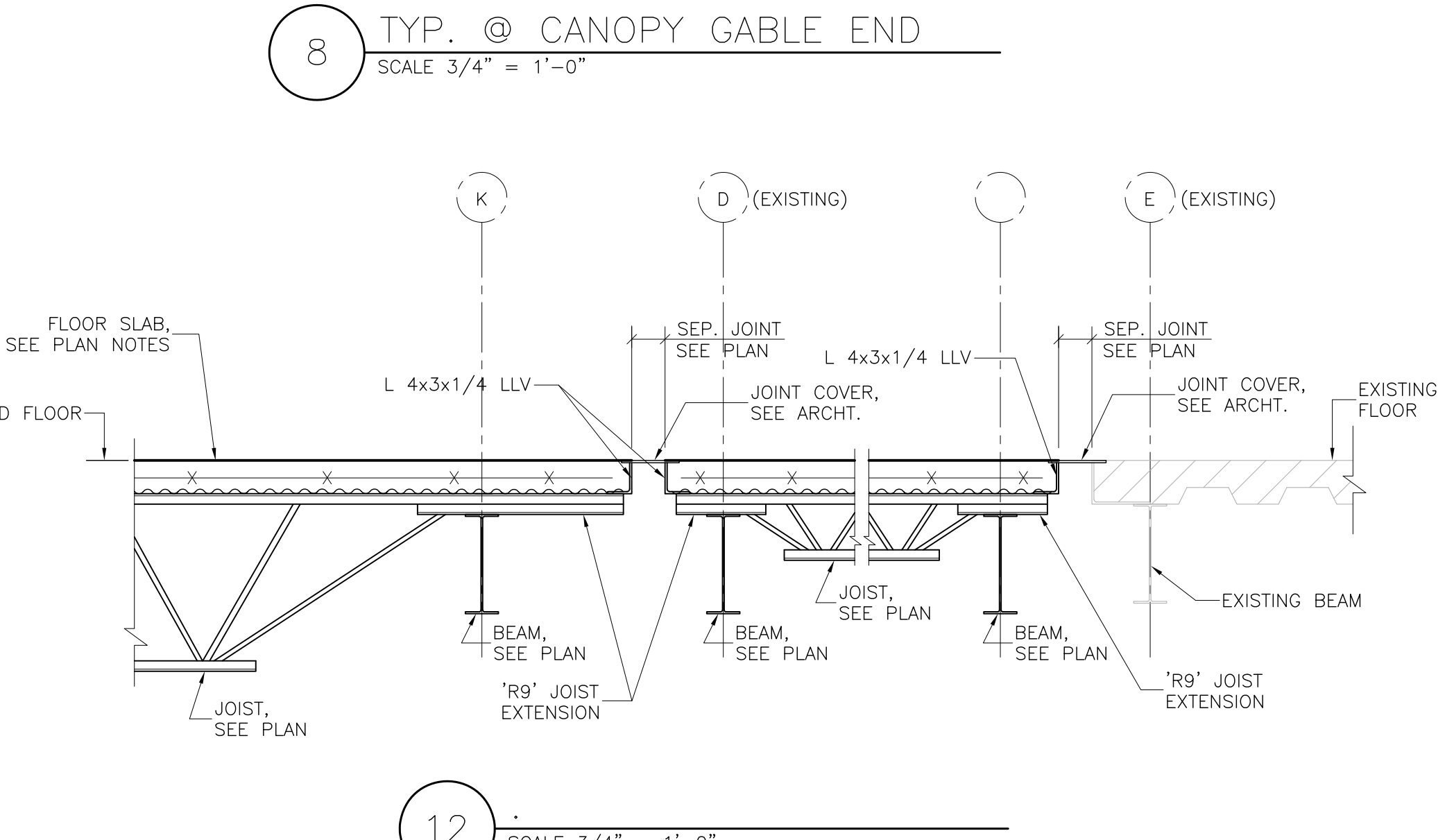
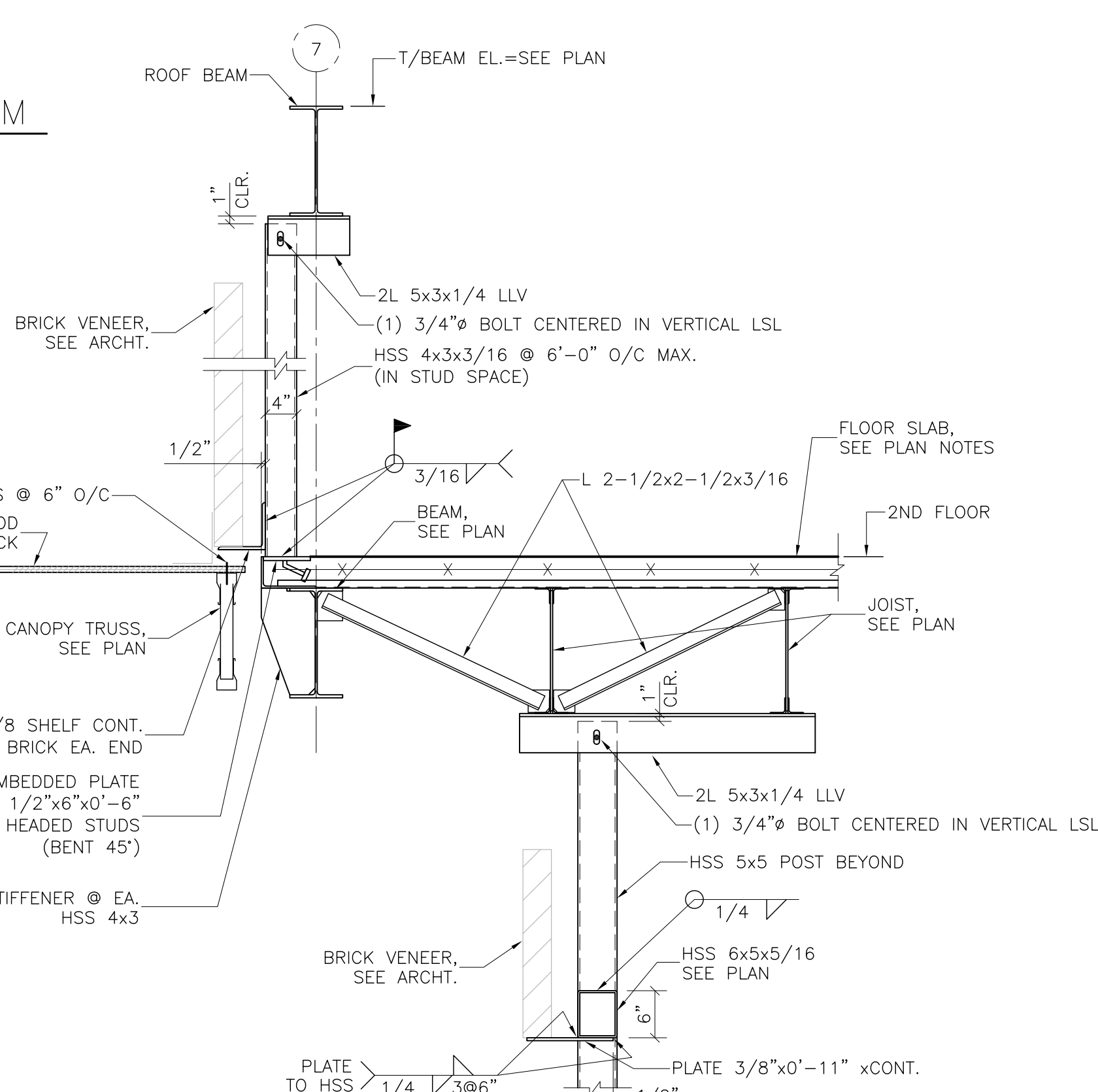
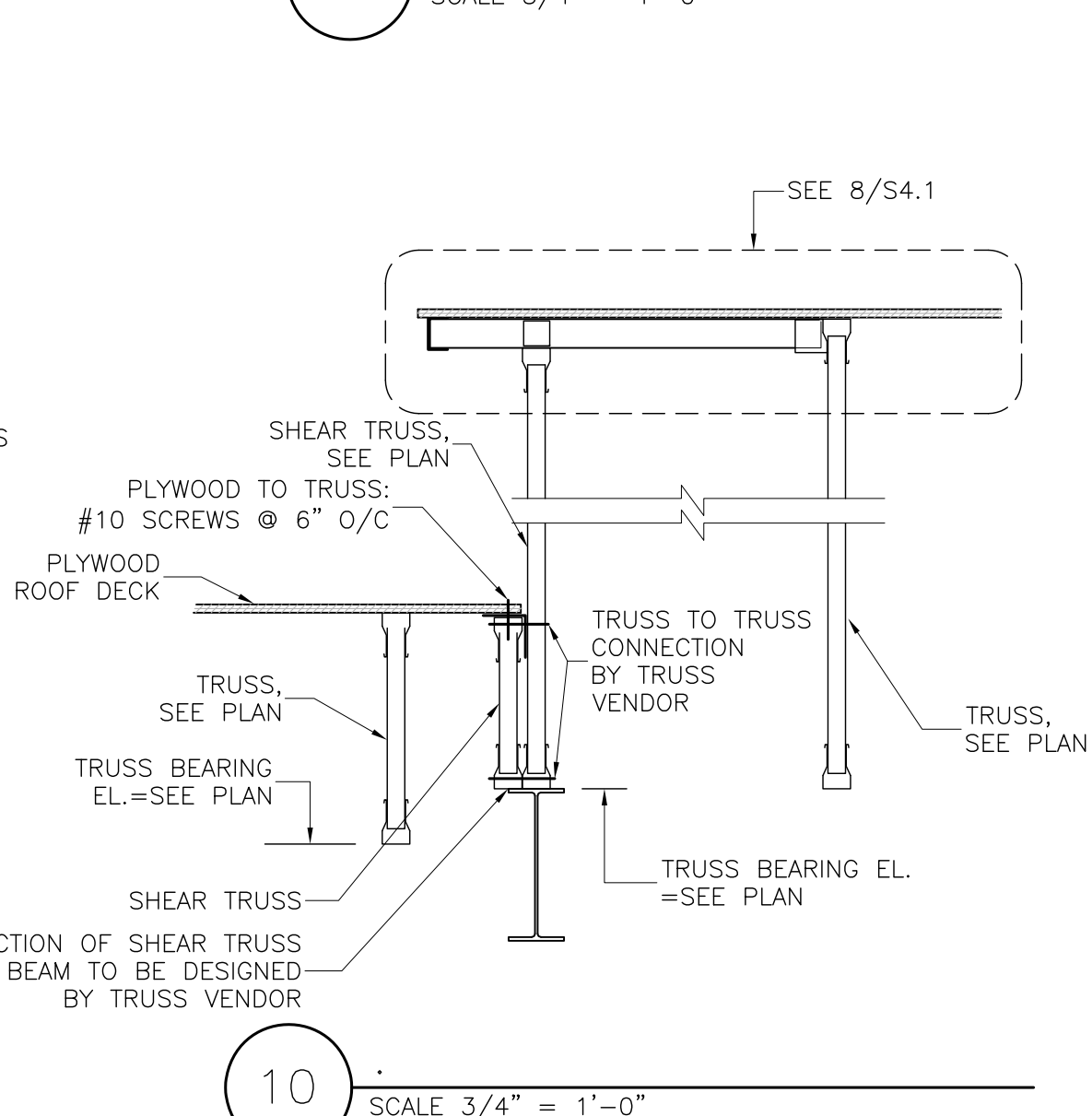
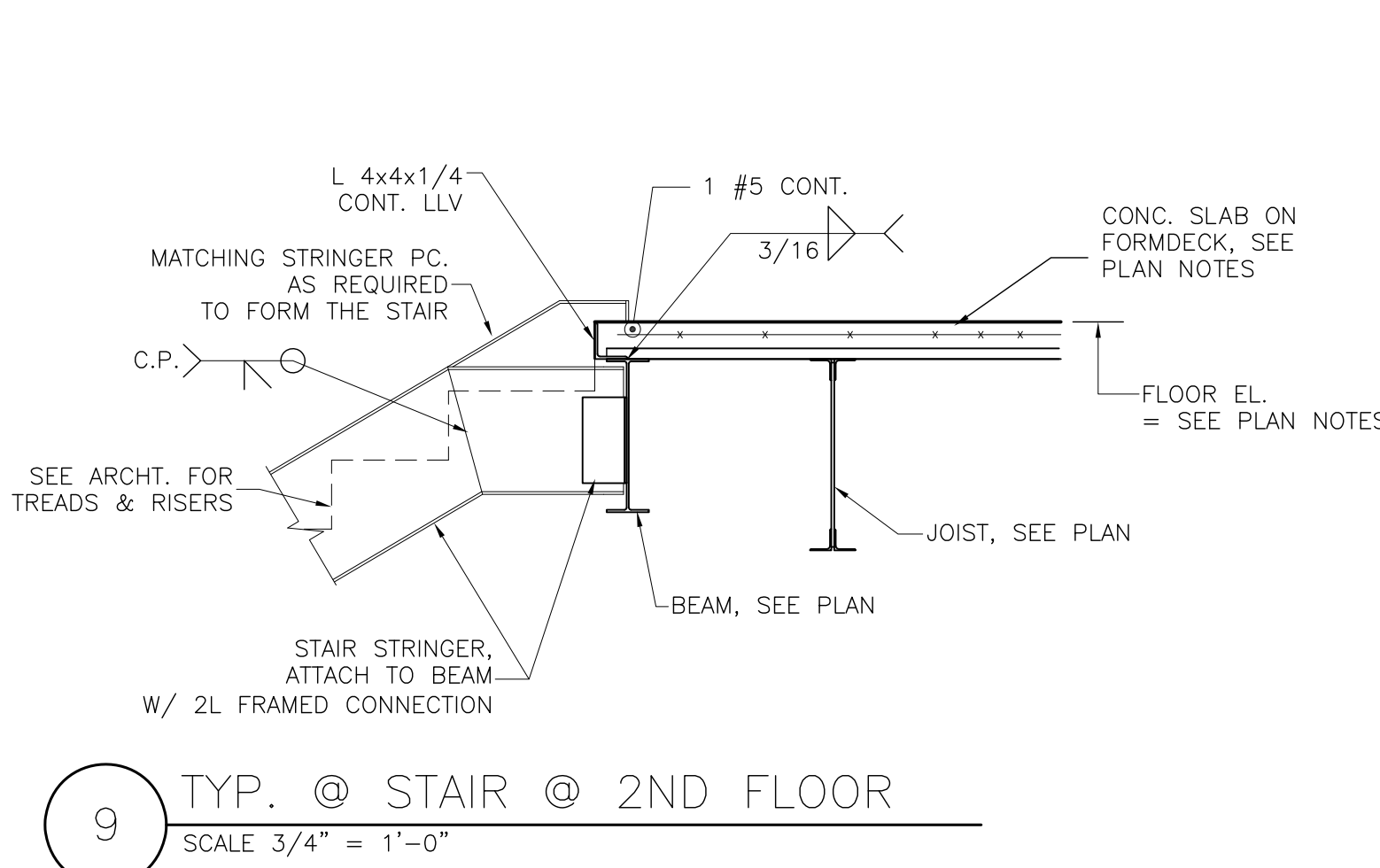
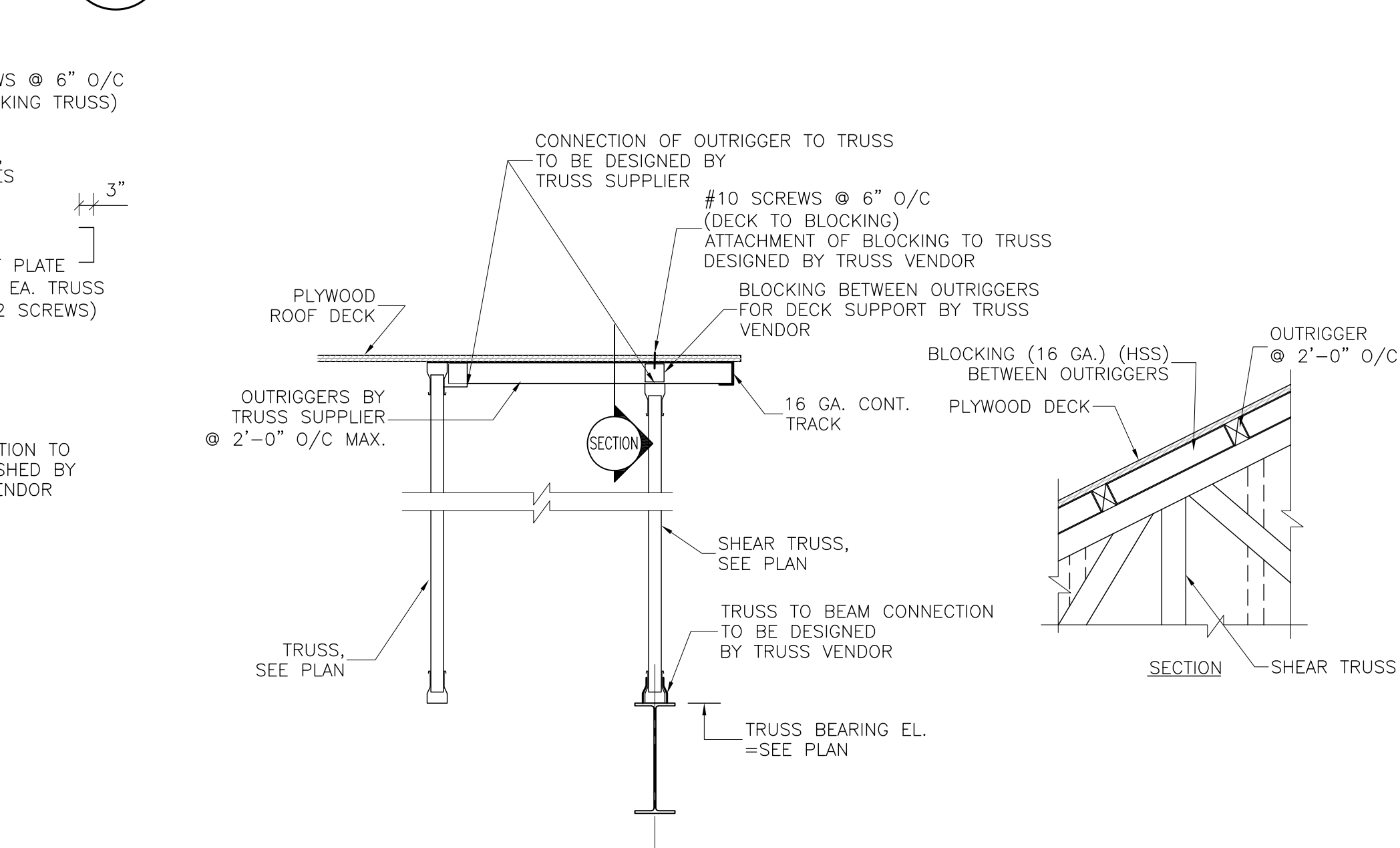
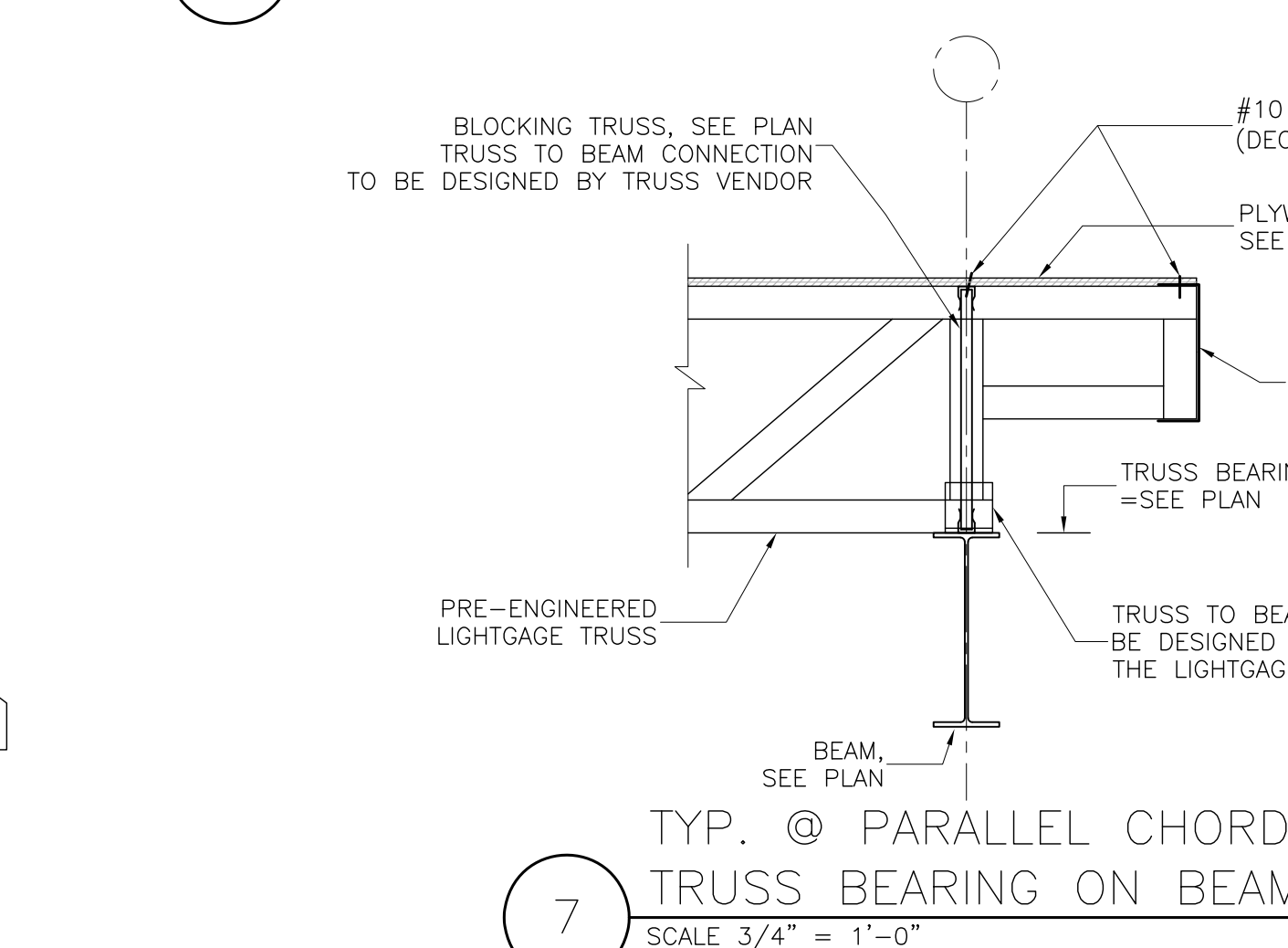
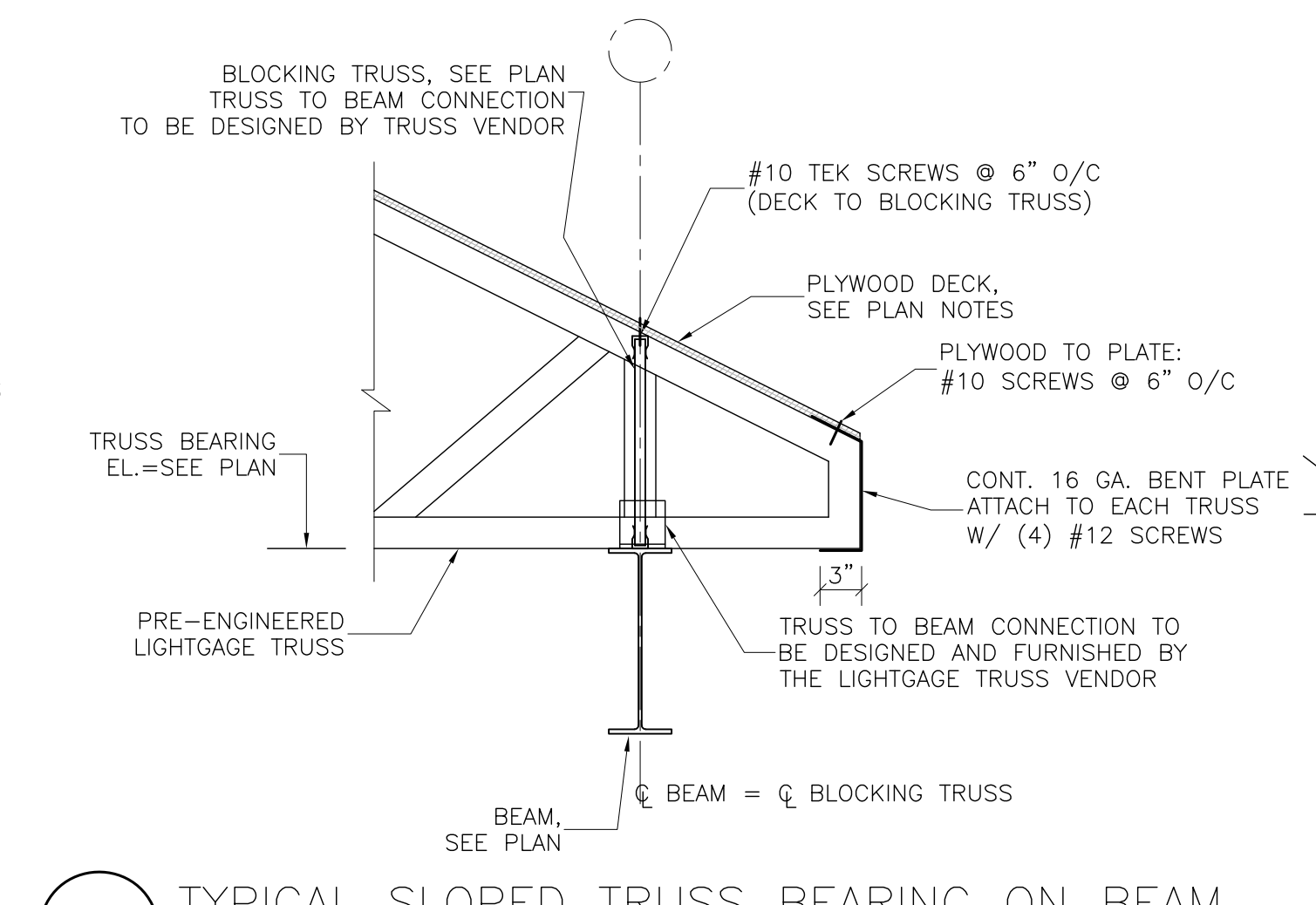
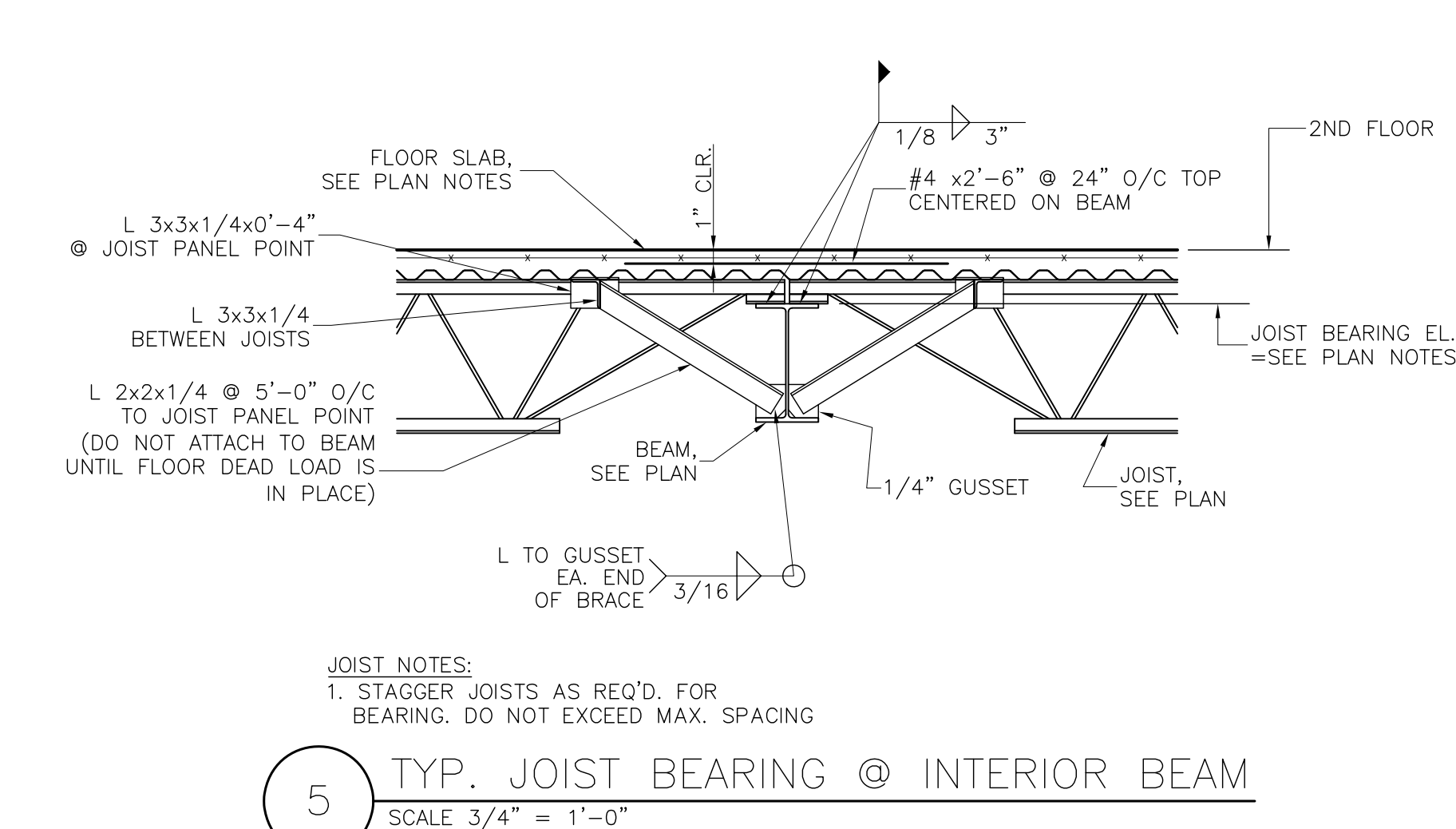
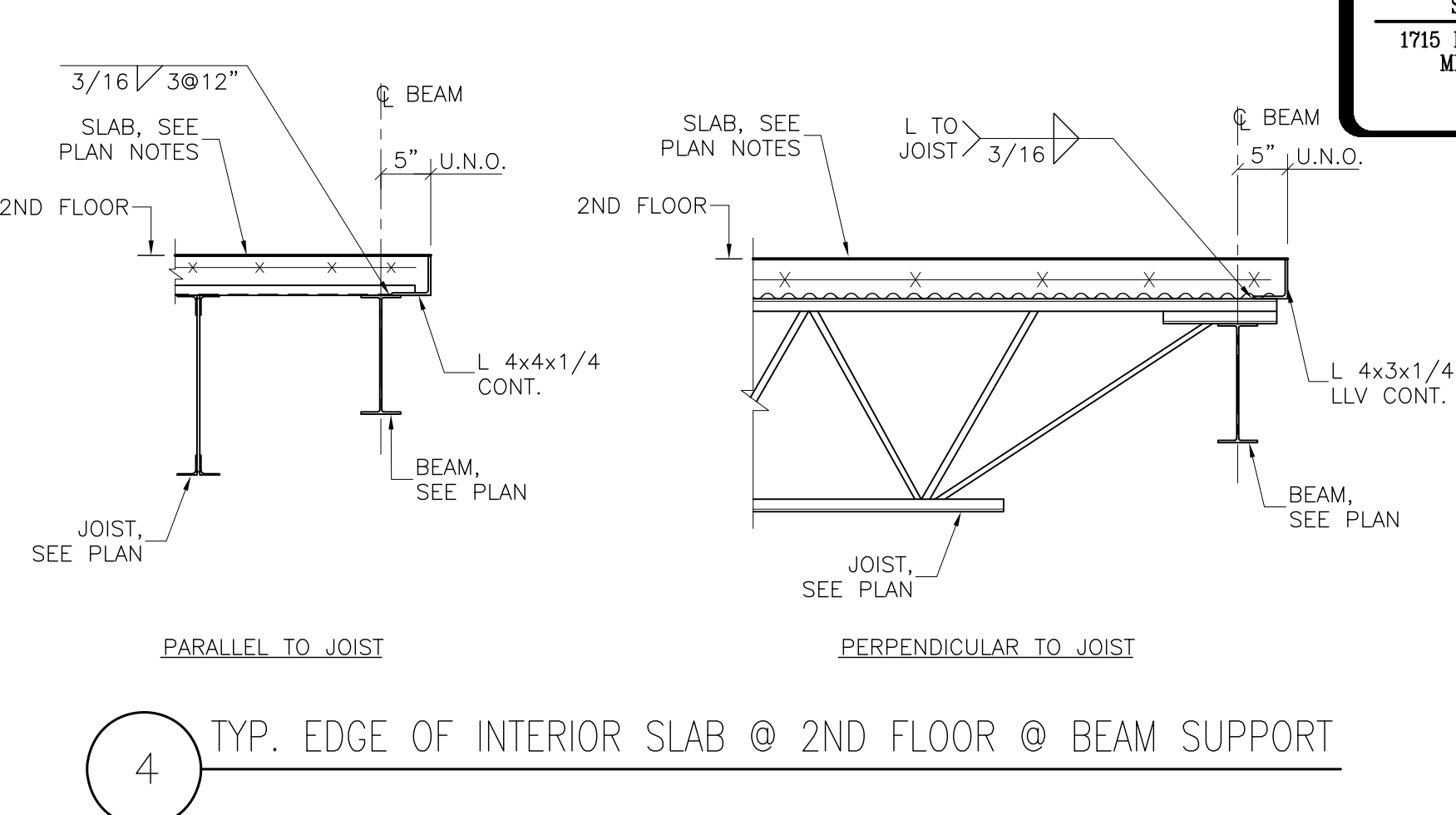
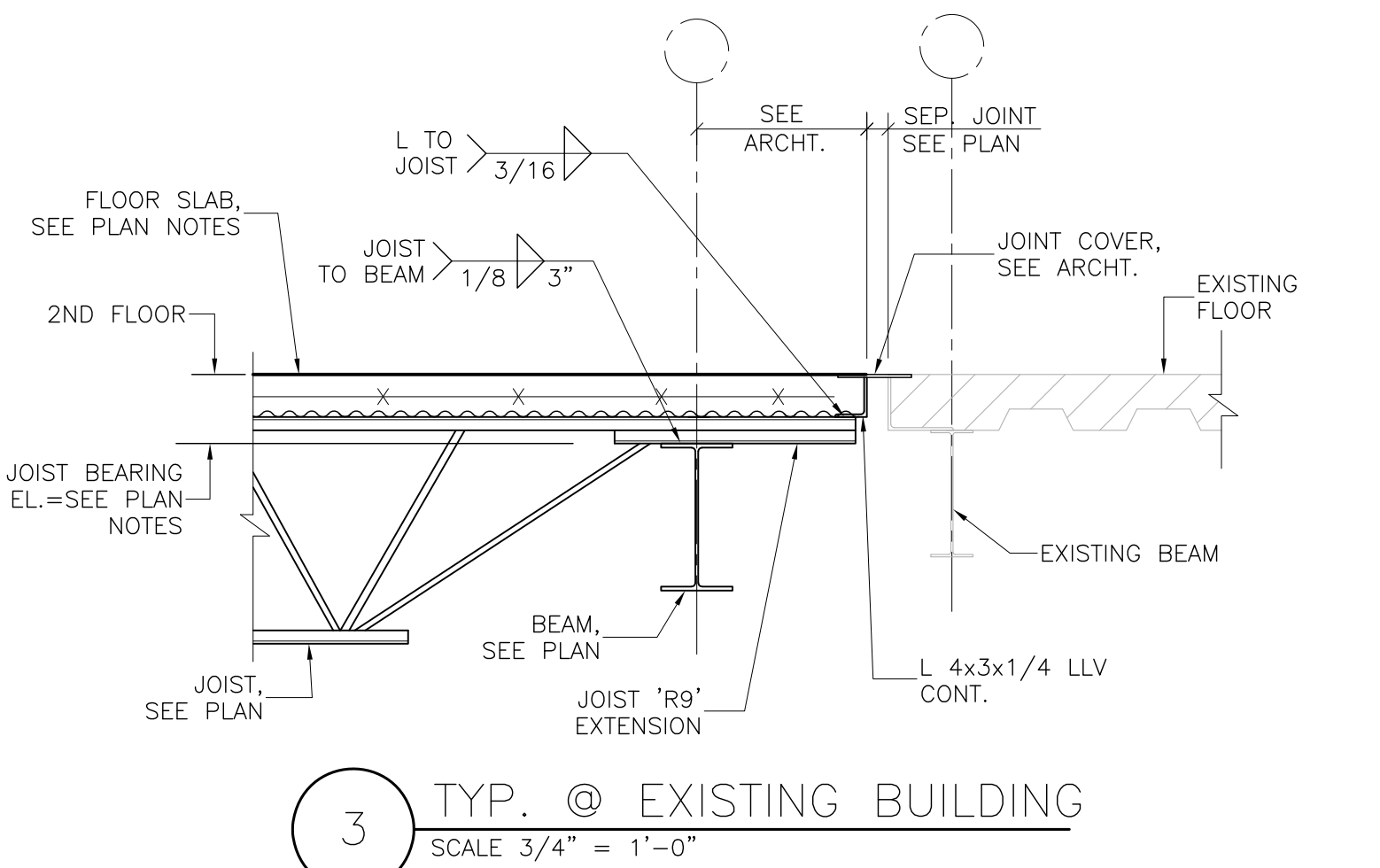
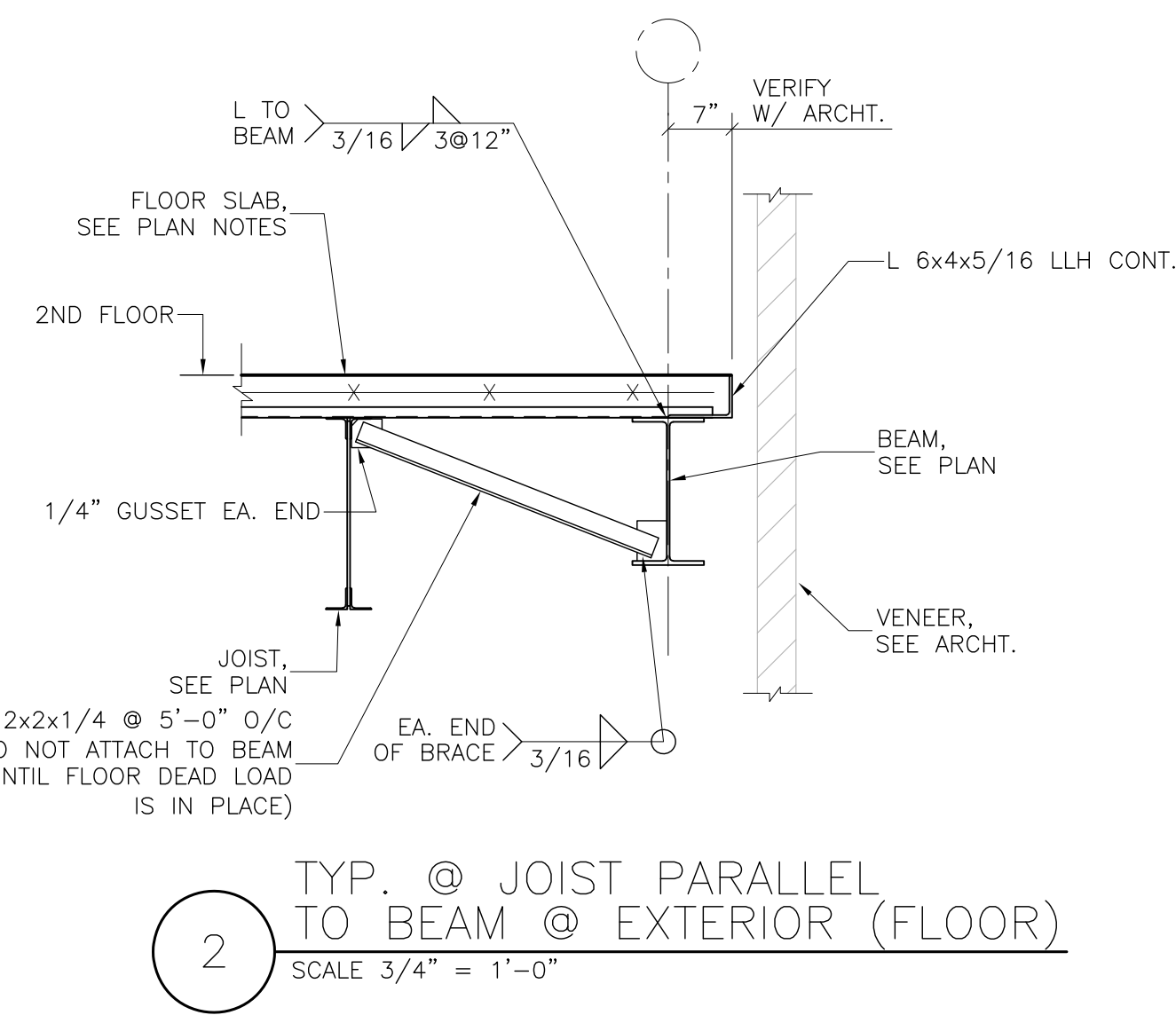
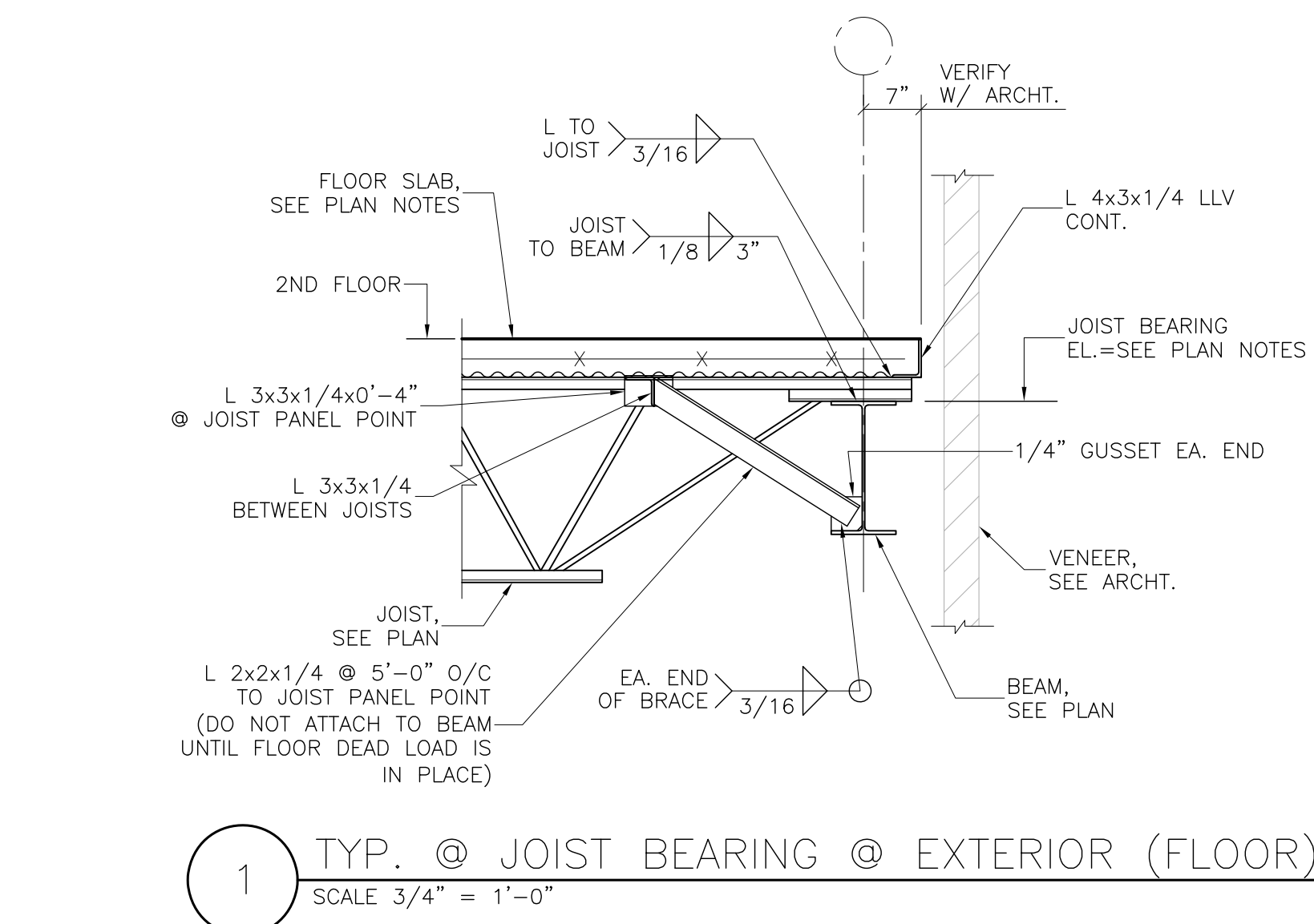
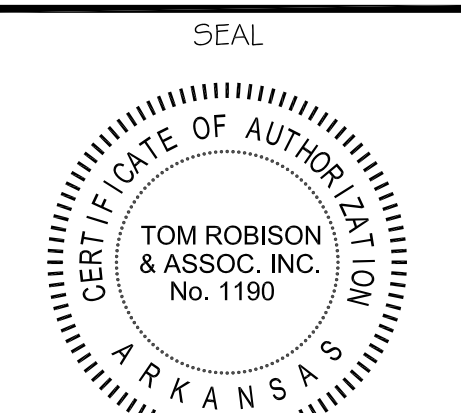
8 SCALE 3/4" = 1'-0"



11 SCALE 3/4" = 1'-0"

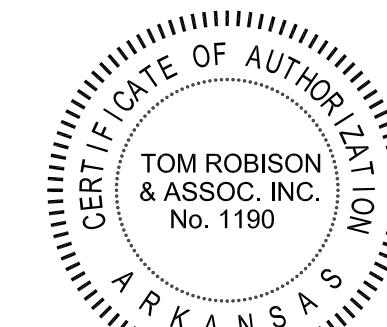


12 TYP. @ EXTERIOR DOOR
SCALE 3/4" = 1'-0"





SEAL



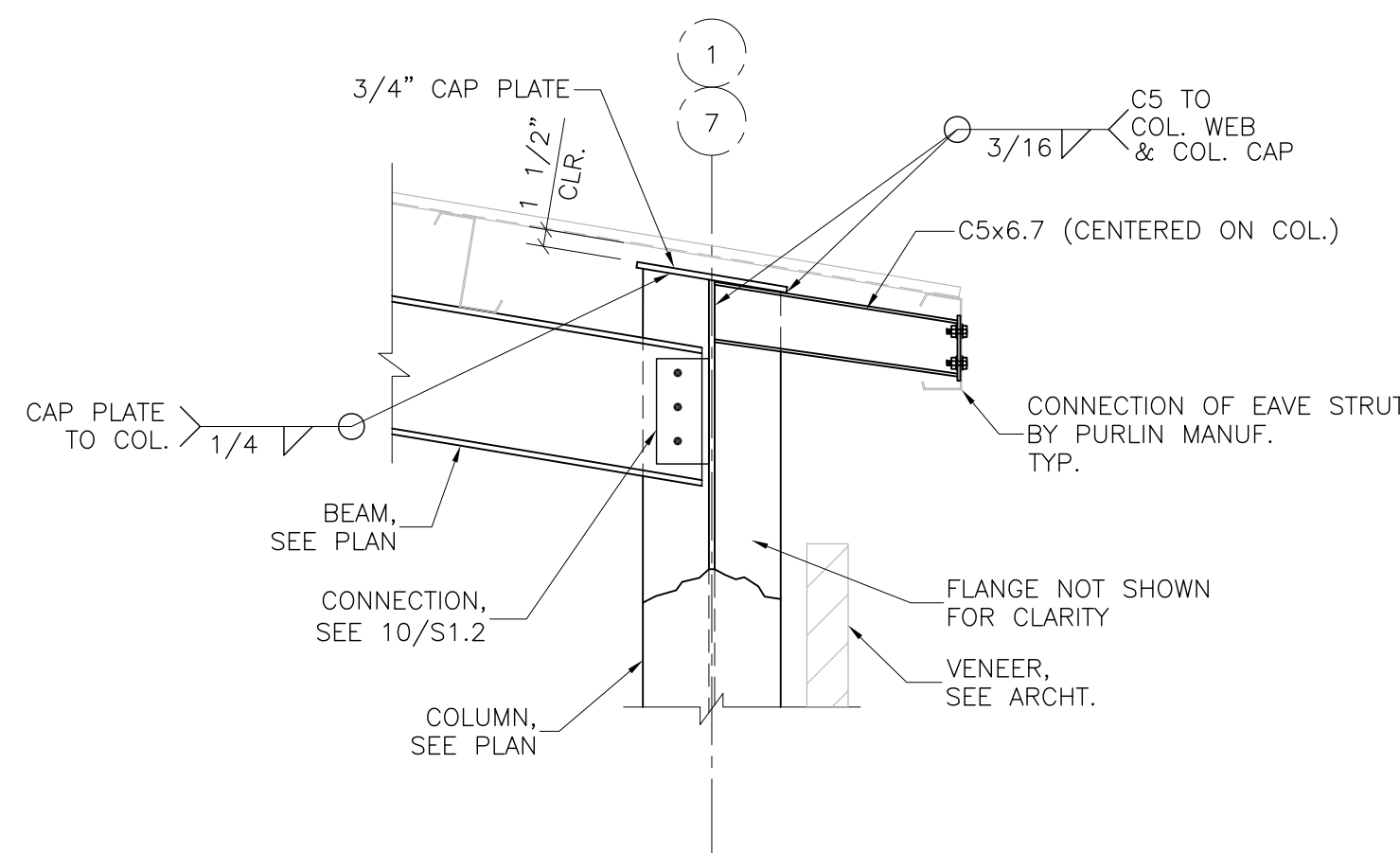
SEAL



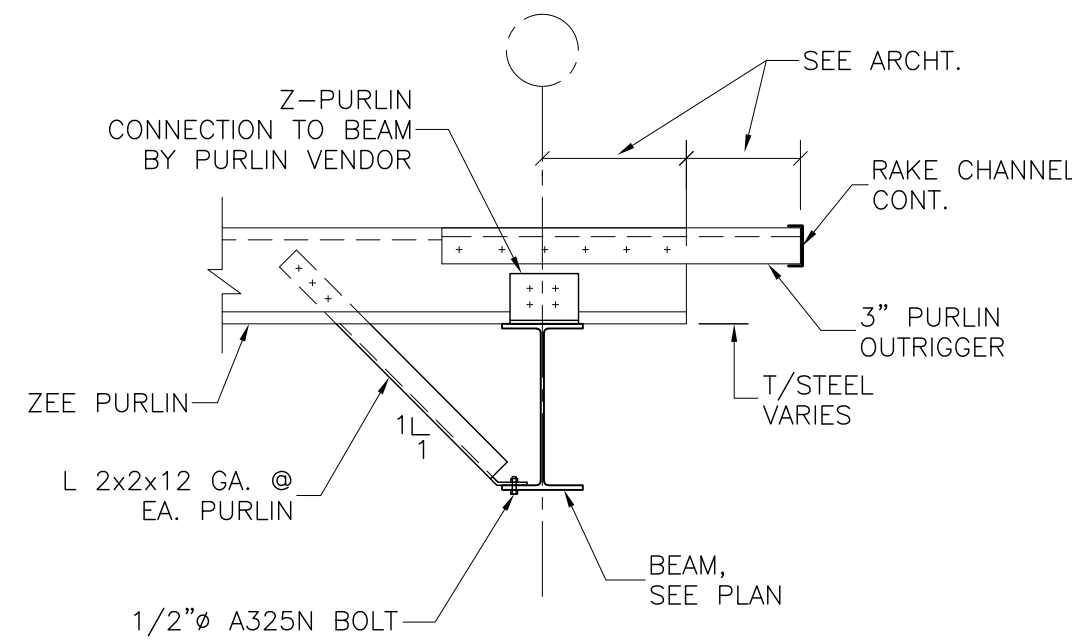
SECTIONS & DETAILS

S5.1

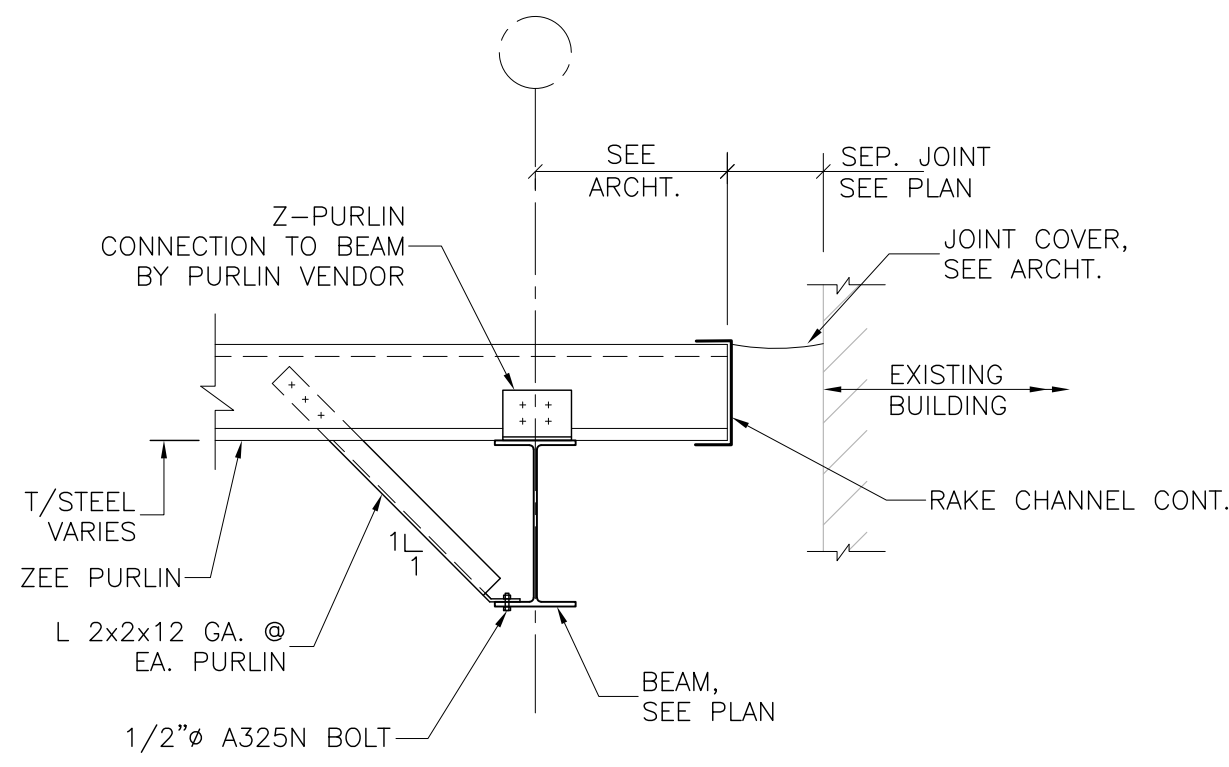
SHEET NO.



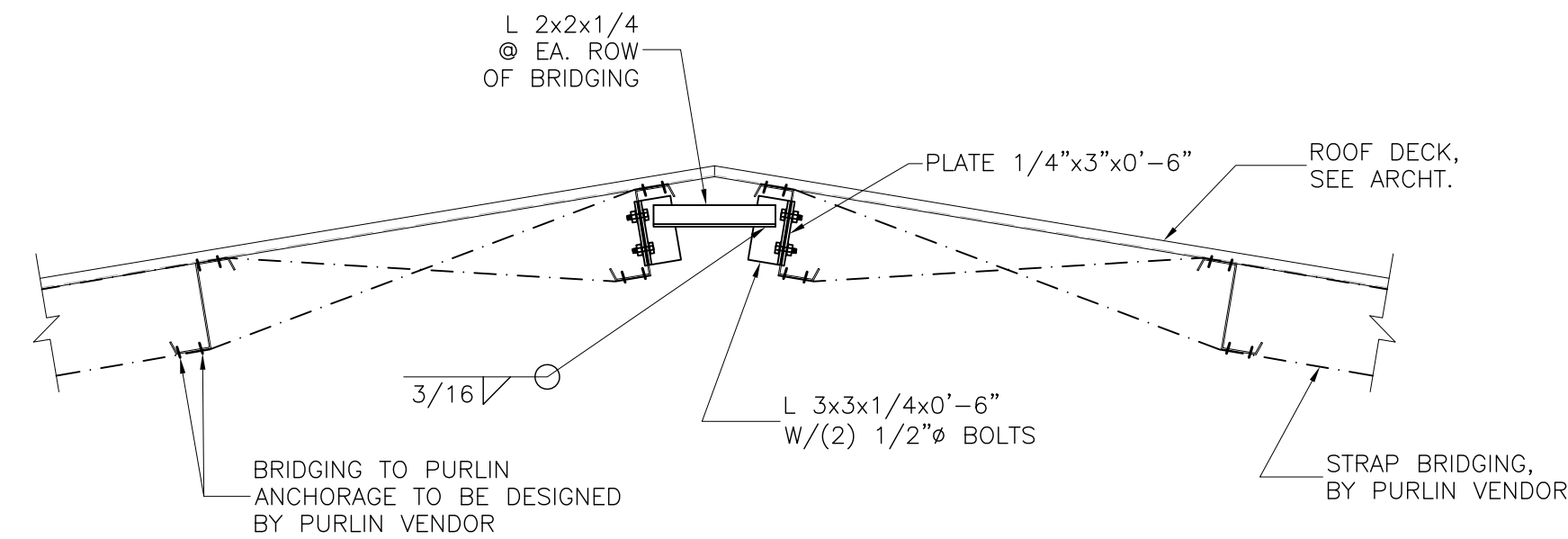
1 TYP. @ EXTERIOR COLUMN
@ ROOF (PARALLEL TO ZEE's)
SCALE 3/4" = 1'-0"



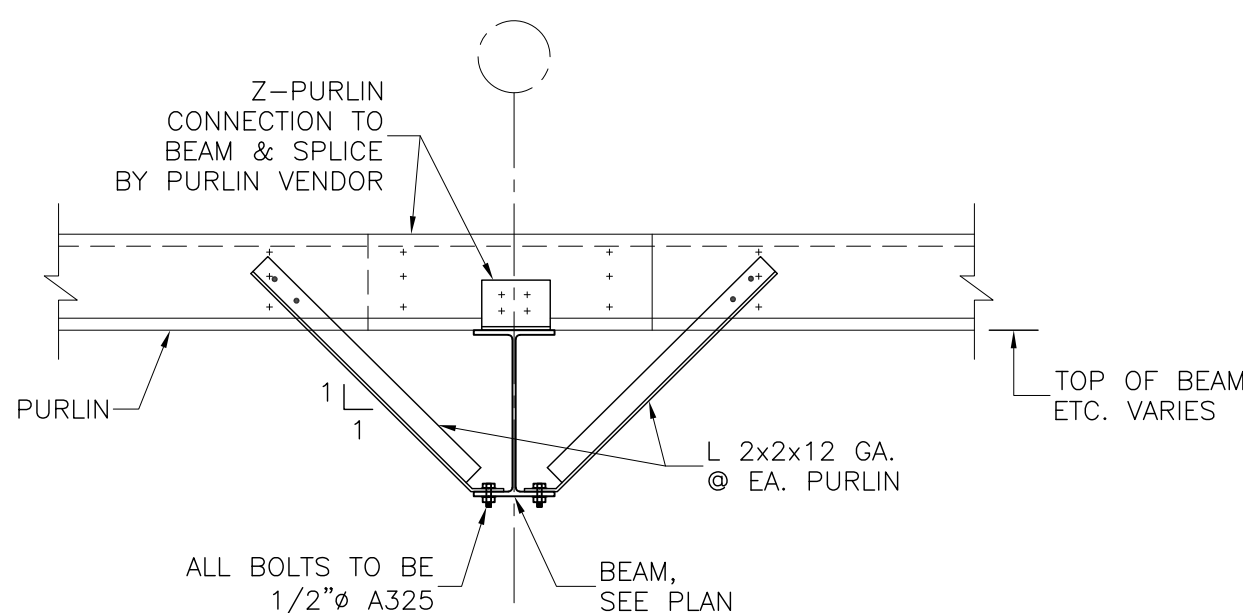
2 TYPICAL @ ROOF RAKE
SCALE 3/4" = 1'-0"



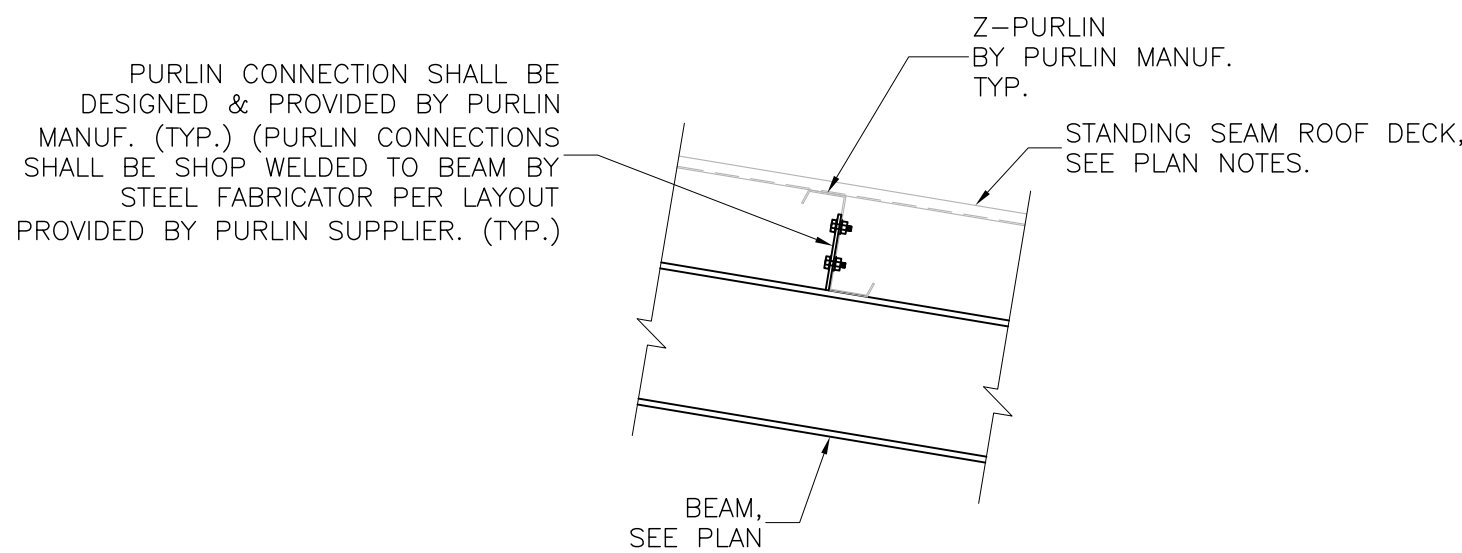
3 TYPICAL @ EXIST. BUILDING
SCALE 3/4" = 1'-0"



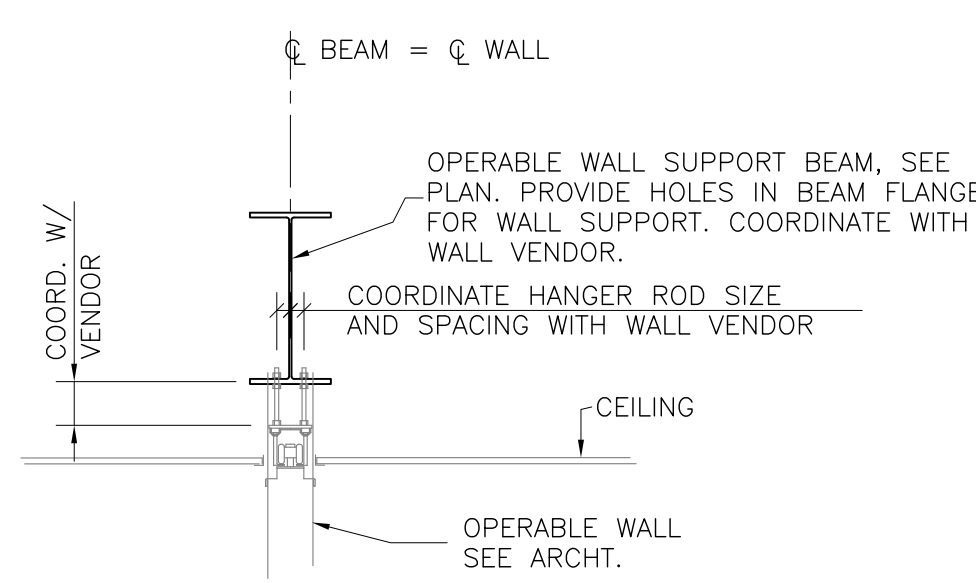
4 TYPICAL @ RIDGE
SCALE 3/4" = 1'-0"
NOTES:
1. BRIDGING SIZE AND SPACING TO BE DESIGNED BY ZEE PURLIN VENDOR.



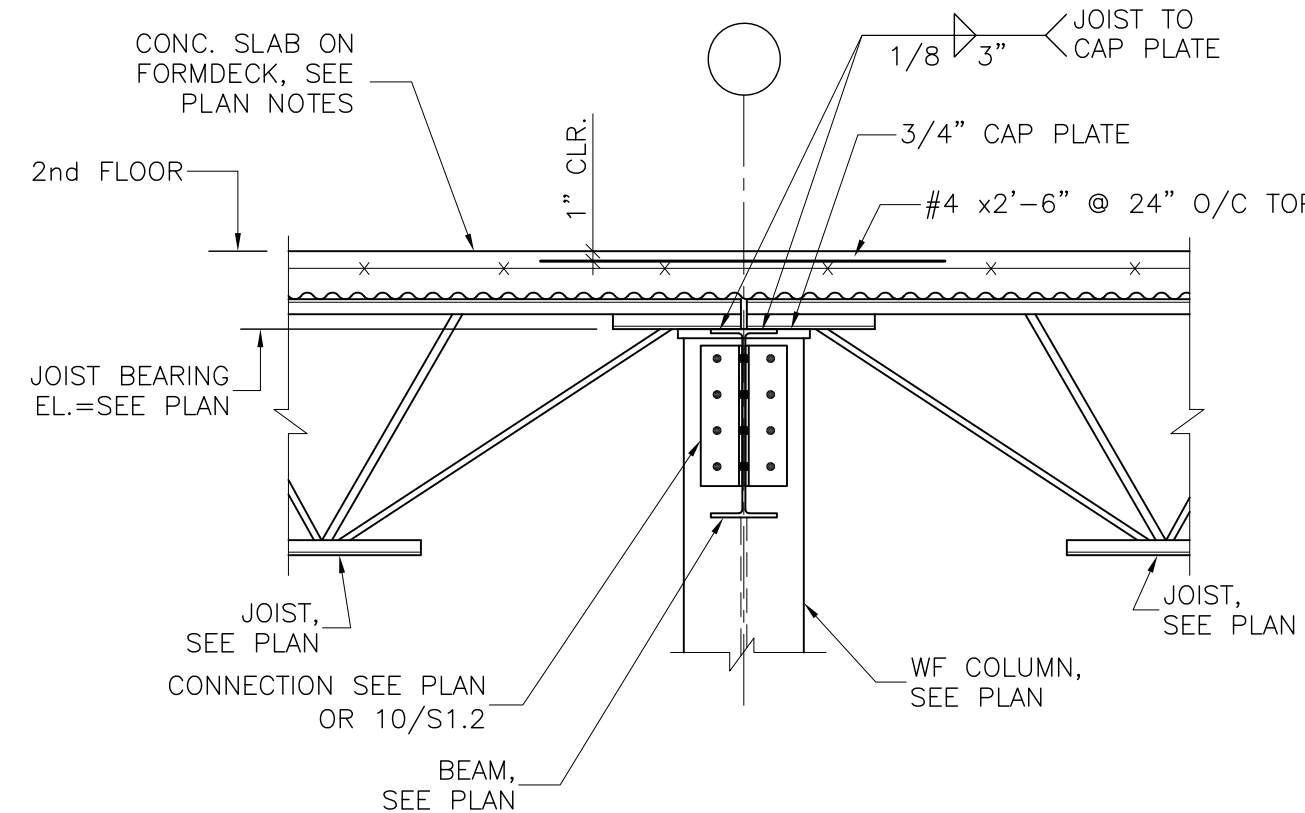
5 TYPICAL @ INTERIOR
SUPPORT OF ZEE PURLINS
SCALE 3/4" = 1'-0"



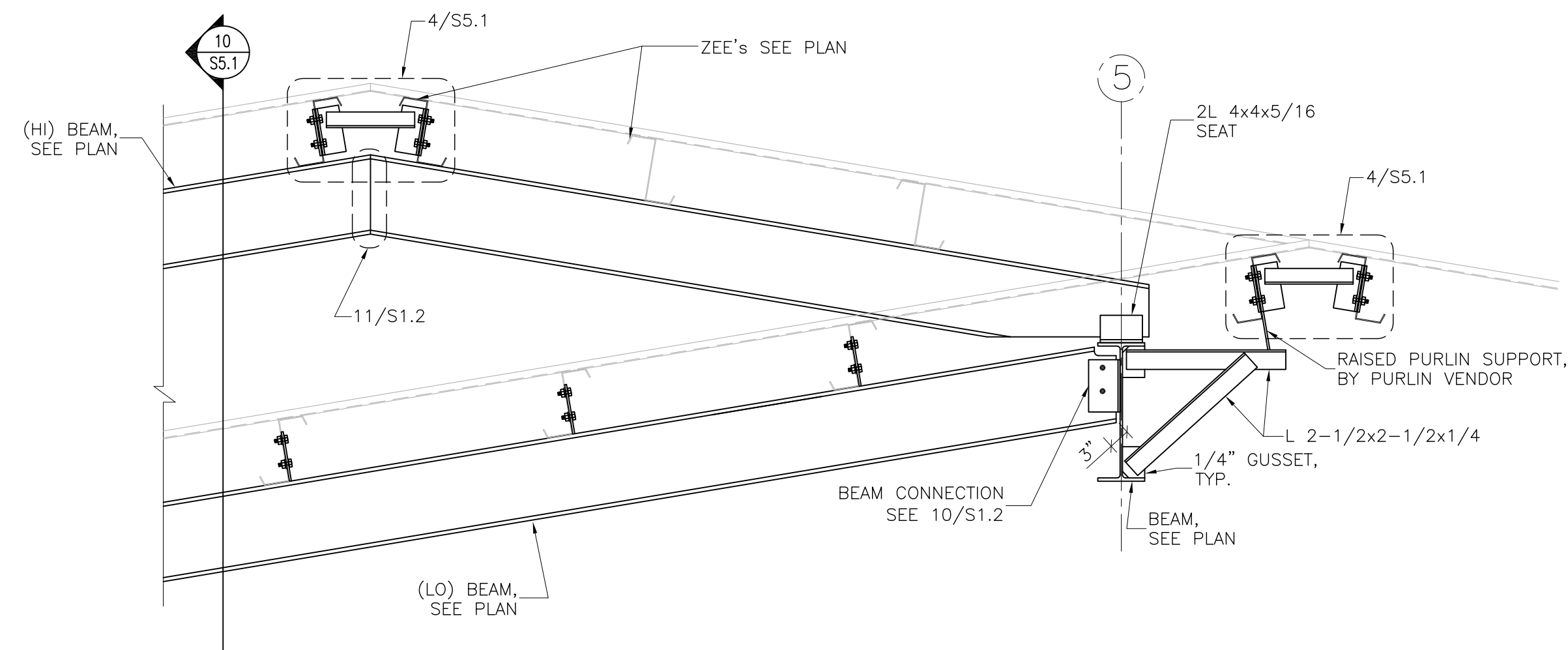
6 TYPICAL PURLIN ATTACHMENT DETAIL
SCALE 3/4" = 1'-0"



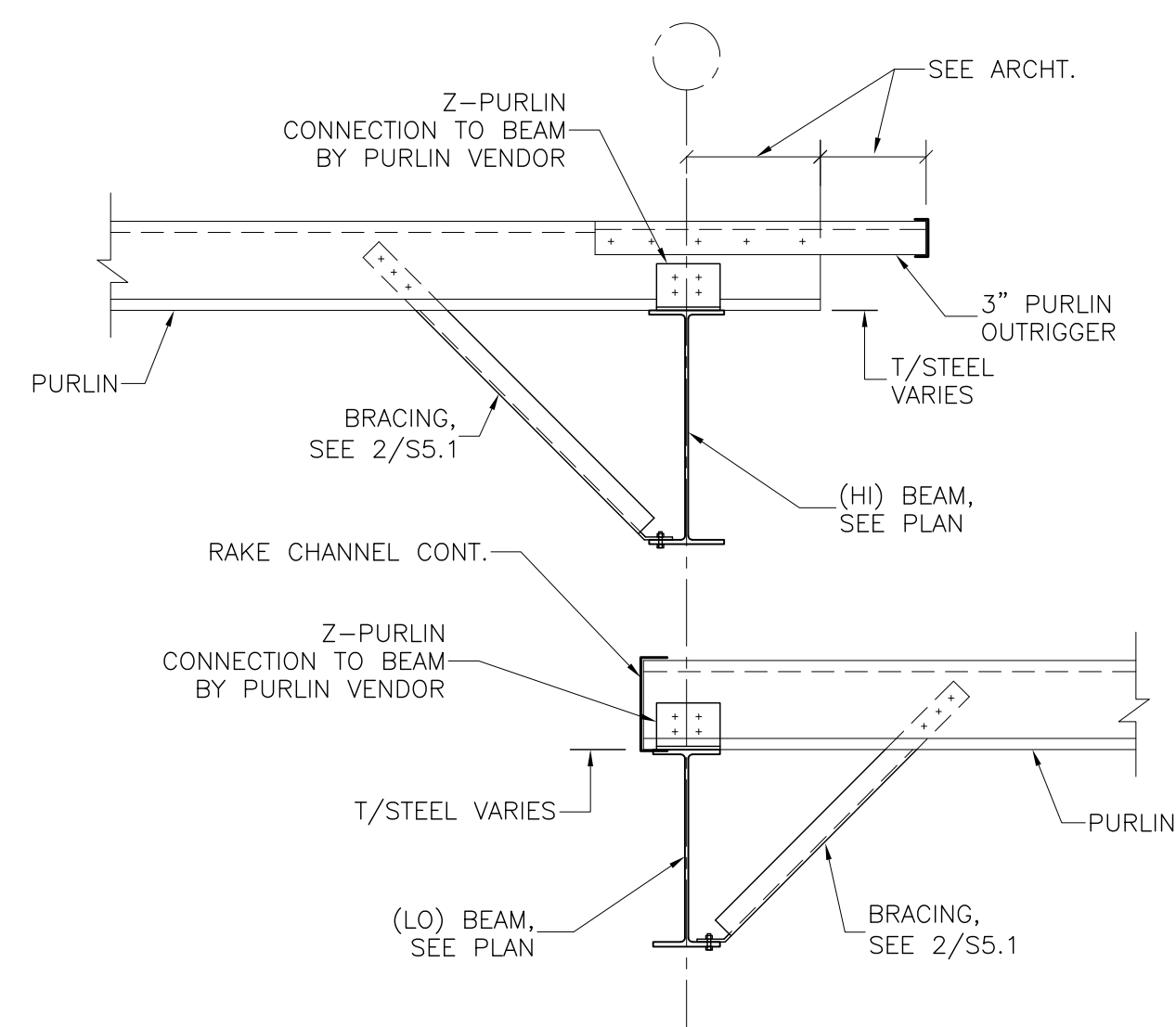
7 TYPICAL SECTION AT OPERABLE WALL
SCALE 3/4" = 1'-0"



8 TYPICAL @ COLUMN STOP @ 2ND FLOOR
SCALE 3/4" = 1'-0"

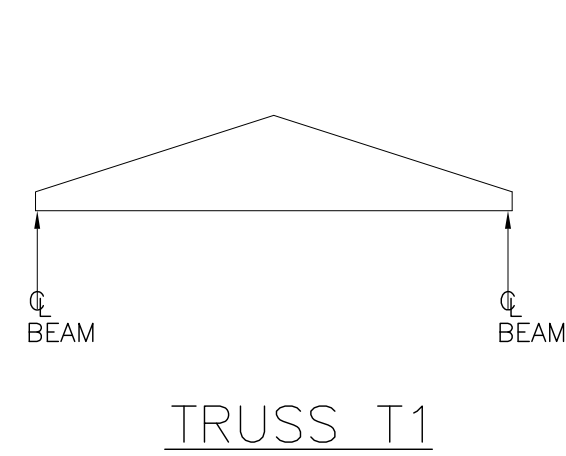


9 SCALE 3/4" = 1'-0"

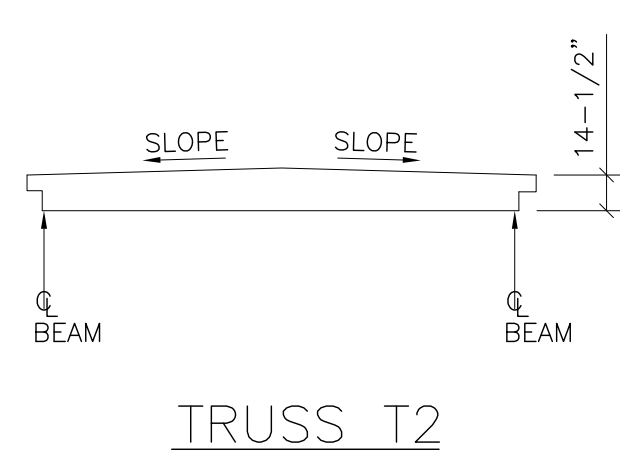


10 SCALE 3/4" = 1'-0"

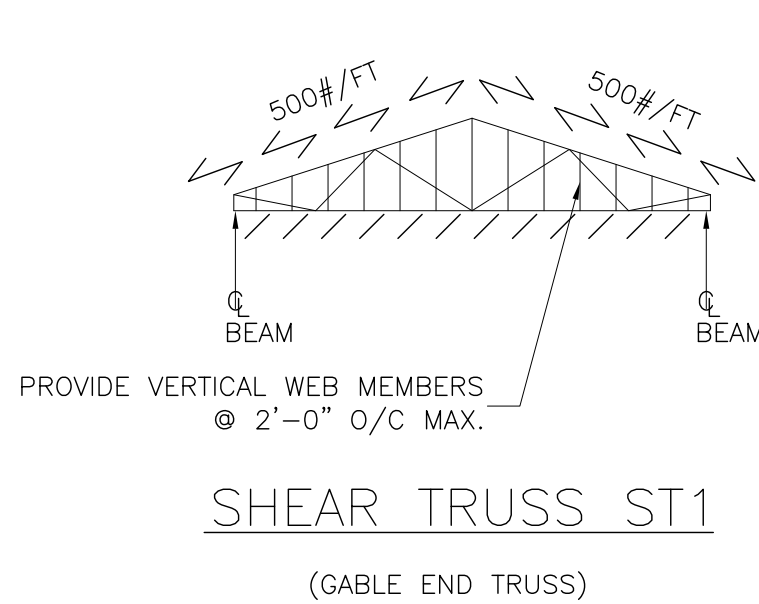
TYP. LIGHT GAGE STEEL TRUSS PROFILES & NOTES



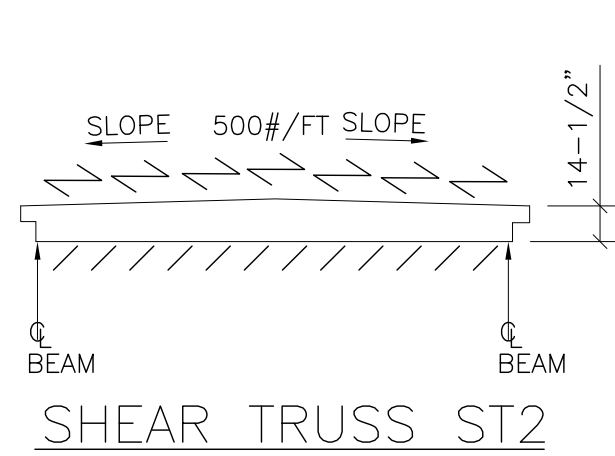
TRUSS T1



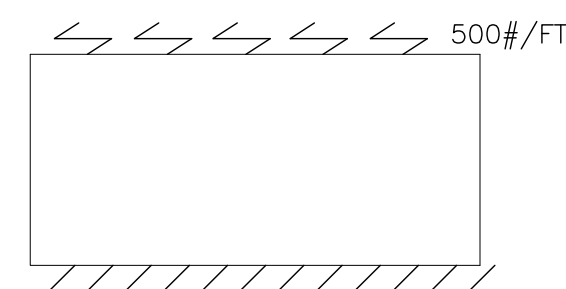
TRUSS T2



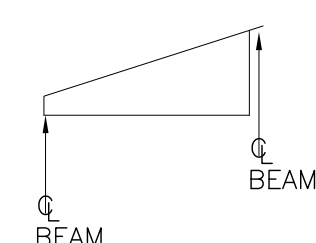
SHEAR TRUSS ST1
(GABLE END TRUSS)



SHEAR TRUSS ST2



BLOCKING TRUSS



TRUSS T3

NOTES:

1. TRUSS AND WALL ELEVATIONS ARE SCHEMATIC ONLY AND ARE INTENDED TO REPRESENT TOP AND BOTTOM CHORD PROFILES AND TYPICAL SUPPORT LOCATIONS ONLY.
2. TRUSS WEB CONFIGURATION AT CONTRACTOR'S OPTION SUBJECT TO ARCHITECT'S/ENGINEER'S APPROVAL. COORDINATE DUCT RUNS THROUGH TRUSS WEB WITH MECH. CONTRACTOR.
3. SEE PLANS AND DETAILS FOR CHORD PROJECTIONS AND RELATED FRAMING
4. ESTABLISH ACTUAL DIMENSIONS FROM ROOF PLAN AND DETAILS. VERIFY ALL DIMENSIONS, ELEVATIONS AND ROOF SLOPES WITH ARCHITECTURAL PLANS, SECTIONS AND DETAILS.
5. SEE FRAMING PLANS & PLAN NOTES FOR DESIGN LOADS.
6. SEE GENERAL NOTES ON SHEET S1.0.
7. LATERAL LOADS ARE SHOWN TO INDICATE UNBALANCED & REVERSIBLE LOADING DUE TO WIND AND/OR SEISMIC FORCES. DESIGN EACH TOP CHORD, ETC. FOR WORST CASE LOAD COMBINATIONS.
8. SUBMIT SHOP DRAWINGS, CALCULATIONS, ETC. SEE SPECS
9. ALL TRUSS TO TRUSS AND TRUSS TO SUPPORT CONNECTIONS SHALL BE DESIGNED, DETAILED AND FURNISHED BY THE TRUSS SUPPLIER AND SUBMITTED ON THE SHOP DRAWINGS FOR A/E APPROVAL, REGARDLESS OF CONNECTION DETAILS OR SIZES SHOWN ON STRUCTURAL DRAWINGS.

10. DESIGN TRUSSES SUPPORTING RTU FOR SPECIFIC RTU AND VVR BOX LOAD AND LOCATION, IN ADDITION TO DEAD+LIVE LOAD. COORDINATE WITH M/P/E CONTRACTORS.
11. DESIGN TRUSS FOR BRICK LOAD. SEE PLANS AND SECTIONS. DEFLECTION OF TRUSS UNDER BRICK LOAD SHALL BE LESS THAN OR EQUAL TO L/600.
12. DESIGN TRUSS BOTTOM CHORD AND ITS CONNECTIONS TO PREVENT LATERAL TORSIONAL BUCKLING OF STEEL BEAM TOP FLANGE SUPPORTING THE TRUSS.
13. PROVIDE PERMANENT BRACING/BRIDGING TO RESIST WIND UPLIFT NOTED IN SPECS AND GENERAL NOTES.
14. TOP CHORD OF ALL TRUSSES AND BOTTOM CHORD OF BLOCKING & SPECIAL SHEAR TRUSS SHALL BE AT LEAST 18 GA. THICK.
15. TRUSSES SHALL BE DESIGNED TO SUPPORT SNOW LOADS FROM DRIFTING AND SLIDING SNOW. THE TRUSS DESIGNER SHALL DETERMINE UNBALANCED SNOW LOADS BASED ON APPLICABLE BUILDING CODES.
16. TRUSS LAYOUT THAT CHANGES LOAD PATH TO THE SUPPORTING STRUCTURE SHALL NOT BE PERMITTED. ADDITIONAL COSTS FOR DESIGN AND CONSTRUCTION DUE TO LOAD PATH MODIFICATION SHALL BE BORNE BY THE TRUSS SUPPLIER.
17. TRUSS AND TRUSS ASSEMBLY STABILITY DURING CONSTRUCTION SHALL BE THE RESPONSIBILITY SOLELY OF THE CONTRACTOR. PROVIDE AND ANCHOR TEMPORARY AND PERMANENT STABILITY BRACING AS REQUIRED.